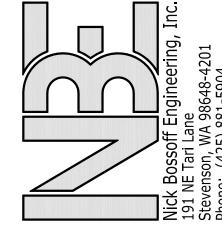
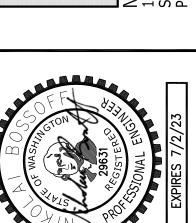


- 1. APPROVAL OF THIS EROSION AND SEDIMENT CONTROL (ESC) PLAN DOES NOT CONSTITUTE AN APPROVAL OF
- 3. THE BOUNDARIES OF THE CLEARING LIMITS SHOWN ON THIS PLAN SHALL BE CLEARLY FLAGGED BY A CONTINUOUS LENGTH OF SURVEY TAPE (OR FENCING, IF REQUIRED) PRIOR TO CONSTRUCTION. DURING THE CONSTRUCTION PERIOD, NO DISTURBANCE BEYOND THE CLEARING LIMITS SHALL BE PERMITTED. THE CLEARING LIMITS SHALL BE MAINTAINED BY THE APPLICANT/ESC SUPERVISOR FOR THE DURATION OF CONSTRUCTION.
- 4. THE ESC FACILITIES SHOWN ON THIS PLAN MUST BE CONSTRUCTED PRIOR TO OR IN CONJUNCTION WITH ALL CLEARING AND GRADING SO AS TO ENSURE THAT THE TRANSPORT OF SEDIMENT TO SURFACE WATERS, DRAINAGE SYSTEMS. AND ADJACENT PROPERTIES IS MINIMIZED.
- 5. THE ESC FACILITIES SHOWN ON THIS PLAN ARE THE MINIMUM REQUIREMENTS FOR ANTICIPATED SITE CONDITIONS. DURING THE CONSTRUCTION PERIOD, THESE ESC FACILITIES SHALL BE UPGRADED AS NEEDED FOR UNEXPECTED STORM EVENTS AND MODIFIED TO ACCOUNT FOR CHANGING SITE CONDITIONS (E.G., ADDITIONAL SUMP PUMPS, RELOCATION OF DITCHES AND SILT FENCES, ETC.).
- 6. THE ESC FACILITIES SHALL BE INSPECTED DAILY BY THE APPLICANT/ESC SUPERVISOR AND MAINTAINED TO ENSURE CONTINUED PROPER FUNCTIONING. WRITTEN RECORDS SHALL BE KEPT OF WEEKLY REVIEWS OF THE ESC FACILITIES DURING THE WET SEASON (OCT. 1 TO APRIL 30) AND OF MONTHLY REVIEWS DURING THE DRY SEASON (MAY 1 TO SEPT. 30).
- 7. ANY AREAS OF EXPOSED SOILS, INCLUDING ROADWAY EMBANKMENTS, THAT WILL NOT BE DISTURBED FOR TWO DAYS DURING THE WET SEASON OR SEVEN DAYS DURING THE DRY SEASON SHALL BE IMMEDIATELY STABILIZED
- WITH THE APPROVED ESC METHODS (E.G., SEEDING, MULCHING, PLASTIC COVERING, ETC.). 8. ANY AREA NEEDING ESC MEASURES NOT REQUIRING IMMEDIATE ATTENTION SHALL BE ADDRESSED WITHIN FIFTEEN
- (15) DAYS. 9. THE ESC FACILITIES ON INACTIVE SITES SHALL BE INSPECTED AND MAINTAINED A MINIMUM OF ONCE A MONTH OR
- WITHIN FORTY-EIGHT (48) HOURS FOLLOWING A STORM EVENT. 10. AT NO TIME SHALL MORE THAN ONE (1) FOOT OF SEDIMENT BE ALLOWED TO ACCUMULATE WITHIN A CATCH BASIN.
- ALL CATCH BASINS AND CONVEYANCE LINES SHALL BE CLEANED PRIOR TO PAVING. THE CLEANING OPERATION SHALL NOT FLUSH SEDIMENT-LADEN WATER INTO THE DOWNSTREAM SYSTEM. 11. STABILIZED CONSTRUCTION ENTRANCES AND ROADS SHALL BE INSTALLED AT THE BEGINNING OF CONSTRUCTION
- AND MAINTAINED FOR THE DURATION OF THE PROJECT. ADDITIONAL MEASURES, SUCH AS WASH PADS, MAY BE REQUIRED TO ENSURE THAT ALL PAVED AREAS ARE KEPT CLEAN FOR THE DURATION OF THE PROJECT. 12. ANY PERMANENT FLOW CONTROL FACILITY USED AS A TEMPORARY SETTLING BASIN SHALL BE MODIFIED WITH THE
- NECESSARY EROSION CONTROL MEASURES AND SHALL PROVIDE ADEQUATE STORAGE CAPACITY. IF THE FACILITY IS TO FUNCTION ULTIMATELY AS AN INFILTRATION SYSTEM, THE TEMPORARY FACILITY MUST BE GRADED SO THAT THE BOTTOM AND SIDES ARE AT LEAST THREE FEET ABOVE THE FINAL GRADE OF THE PERMANENT FACILITY.
- 13. WHERE STRAW MULCH FOR TEMPORARY EROSION CONTROL IS REQUIRED, IT SHALL BE APPLIED AT A MINIMUM THICKNESS OF 2 TO 3 INCHES.
- 14. PRIOR TO THE BEGINNING OF THE WET SEASON (OCT. 1), ALL DISTURBED AREAS SHALL BE REVIEWED TO IDENTIFY WHICH ONES CAN BE SEEDED IN PREPARATION FOR THE WINTER RAINS. DISTURBED AREAS SHALL BE SEEDED WITHIN ONE WEEK OF THE BEGINNING OF THE WET SEASON. A SKETCH MAP OF THOSE AREAS TO BE SEEDED AND THOSE AREAS TO REMAIN UNCOVERED SHALL BE SUBMITTED TO THE DDES INSPECTOR. THE DDES INSPECTOR CAN REQUIRE SEEDING OF ADDITIONAL AREAS IN ORDER TO PROTECT SURFACE WATERS, ADJACENT PROPERTIES, OR DRAINAGE FACILITIES.

- PLACE TIGHT-FITTING LIDS ON ALL CONTAINERS. 4. ENCLOSE OR COVER THE CONTAINERS WHERE THEY ARE STORED TO PROTECT FROM RAIN. THE LOCAL FIRE DISTRICT MUST BE
- CONSULTED FOR LIMITATIONS ON CLEARANCE OF ROOF COVERS OVER CONTAINERS USED TO STORE FLAMMABLE MATERIALS. 5. RAISE THE CONTAINERS OFF THE GROUND BY USING A SPILL CONTAINMENT PALLET OR SIMILAR METHOD THAT HAS PROVISIONS FOR SPILL CONTROL.
- 6. PLACE DRIP PANS OR ABSORBENT MATERIALS BENEATH ALL MOUNTED CONTAINER TAPS, AND AT ALL POTENTIAL DRIP AND SPILL LOCATIONS DURING FILLING AND UNLOADING OF CONTAINERS. ANY COLLECTED LIQUIDS OR SOILED ABSORBENT MATERIALS MUST BE REUSED, RECYCLED, OR PROPERLY DISPOSED OF.
- STORE AND MAINTAIN ABSORBENT PADS OR APPROPRIATE SPILL CLEANUP MATERIALS NEAR THE CONTAINER STORAGE AREA, IN A LOCATION KNOWN TO ALL. ENSURE THAT EMPLOYEES ARE FAMILIAR WITH THE SITE'S SPILL PLAN AND/OR PROPER SPILL CLEANUP PROCEDURES.
- 8. CHECK CONTAINERS (AND ANY CONTAINMENT SUMPS) DAILY FOR LEAKS AND SPILLS. REPLACE CONTAINERS THAT ARE LEAKING, CORRODED, OR OTHERWISE DETERIORATING. IF THE LIQUID CHEMICALS ARE CORROSIVE, CONTAINERS MADE OF COMPATIBLE MATERIALS MUST BE USED INSTEAD OF METAL DRUMS. NEW OR SECONDARY CONTAINERS MUST BE LABELED WITH THE PRODUCT NAME AND HAZARDS.
- 9. PLACE DRIP PANS OR ABSORBENT MATERIALS BENEATH A CONTAINER THAT IS FOUND TO BE LEAKING. REMOVE THE DAMAGED CONTAINER AS SOON AS POSSIBLE. MOP UP THE SPILLED LIQUID WITH ABSORBENT PADS OR RAGS. ANY COLLECTED LIQUIDS OR SOILED ABSORBENT MATERIALS MUST BE REUSED, RECYCLED, OR PROPERLY DISPOSED OF.
- 1. LOCATE THE FUELING OPERATION TO ENSURE LEAKS OR SPILLS WILL NOT DISCHARGE, FLOW, OR BE WASHED INTO THE STORM
- DRAINAGE SYSTEM. SURFACE WATER. OR GROUNDWATER. USE DRIP PANS OR ABSORBENT PADS TO CAPTURE DRIPS OR SPILLS DURING FUELING OPERATIONS.
- 3. IF FUELING IS DONE DURING EVENING HOURS, LIGHTING MUST BE PROVIDED.
- 4. STORE AND MAINTAIN APPROPRIATE SPILL CLEANUP MATERIALS IN THE MOBILE FUELING VEHICLE. ENSURE THAT EMPLOYEES ARE FAMILIAR WITH PROPER SPILL CONTROL AND CLEANUP PROCEDURES.
- IMMEDIATELY MOP UP ANY SPILLED FUEL WITH ABSORBENT PADS OR RAGS. ANY COLLECTED LIQUIDS OR SOILED ABSORBENT MATERIALS MUST BE REUSED, RECYCLED, OR PROPERLY DISPOSED OF.
- CONCRETE SAW CUTTING, SLURRY, AND WASHWATER DISPOSAL SLURRY FROM SAW CUTTING THE SIDEWALK SHALL BE VACUUMED SO THAT IT DOES NOT ENTER NEARBY STORM DRAINS. 2. CONCRETE TRUCK CHUTES, PUMPS, AND INTERNALS SHALL BE WASHED OUT ONLY INTO FORMED AREAS AWAITING INSTALLATION
- OF CONCRETE. 3. UNUSED CONCRETE REMAINING IN THE TRUCK AND PUMP SHALL BE RETURNED TO THE ORIGINATING BATCH PLANT FOR
- RECYCLING.
- 4. HAND TOOLS INCLUDING, BUT NOT LIMITED, SCREEDS, SHOVELS, RAKES, FLOATS, AND TROWELS SHALL BE WASHED OFF ONLY INTO FORMED INTO FORMED AREAS AWAITING INSTALLATION OF CONCRETE OR IMPERMEABLE ASPHALT.
- 5. EQUIPMENT THAT CANNOT BE EASILY MOVED, SUCH AS CONCRETE PAVERS, SHALL ONLY BE WASHED IN AREAS THAT DO NOT DIRECTLY DRAIN TO NATURAL OR CONSTRUCTED STORMWATER CONVEYANCES.
- WASHDOWN FROM AREAS SUCH AS CONCRETE AGGREGATE DRIVEWAY SHALL NOT DRAIN DIRECTLY TO NATURAL OR CONSTRUCTED WHEN NO FORMED AREAS ARE AVAILABLE, WASHWATER AND LEFTOVER PRODUCT SHALL BE CONTAINED IN A LINED CONTAINER.
- CONTAINED CONCRETE SHALL BE DISPOSED OF IN A MANNER THAT DOES NOT VIOLATE GROUNDWATER OR SURFACE WATER

8. CONTAINERS SHALL BE CHECKED FOR HOLES IN THE LINER DAILY DURING CONCRETE POURS AND REPLACED THE SAME DAY.





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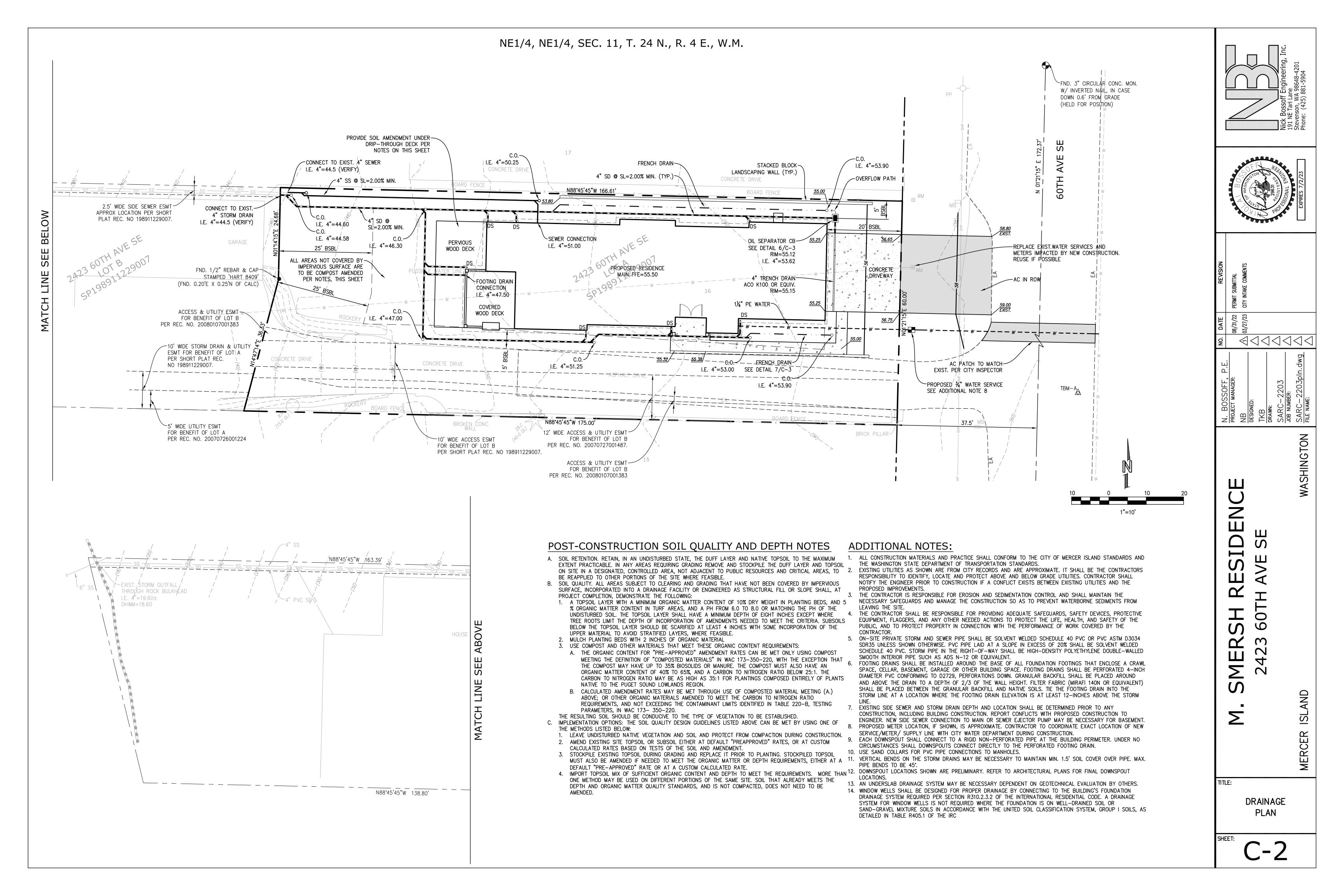
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T.E.S.C. PLAN

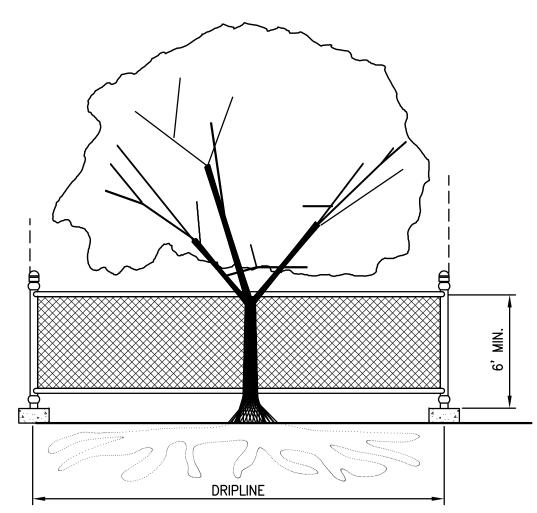
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1-800-424-5555



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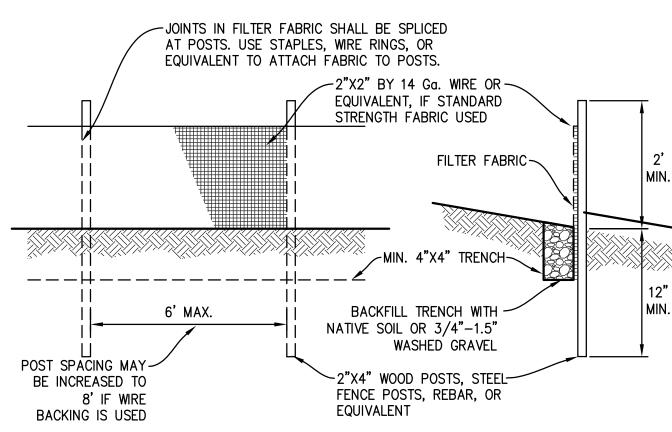


TREE PROTECTION DURING CONSTRUCTION

- 1. 6-FT. HIGH TEMPORARY CHAIN LINK FENCE SHALL BE PLACED AT THE DRIPLINE OF THE TREE TO BE SAVED. FENCE SHALL COMPLETELY ENCIRCLE THE TREE(S). INSTALL FENCE POSTS USING PIER BLOCKS ONLY. AVOID DRIVING POSTS OR STAKES INTO MÁJOR ROOTS.
- 2. FOR ROOTS OVER 1-IN DIA. THAT ARE DAMAGED DURING CONSTRUCTION, MAKE A CLEAN, STRAIGHT CUT TO REMOVE THE DAMAGED PORTION. ALL EXPOSED ROOTS SHALL BE TEMPORARILY COVERED WITH DAMP BURLAP TO PREVENT DRYING, AND SHALL BE COVERED WITH SOIL AS SOON AS POSSIBLE.
- 3. WORK WITHIN PROTECTION FENCE SHALL BE DONE MANUALLY. NO STOCKPILING OF MATERIALS, VEHICULAR TRAFFIC, OR STORAGE OF EQUIPMENT OR MACHINERY SHALL BE ALLOWED WITHIN THE LIMIT OF THE FENCING.

TREE PROTECTION

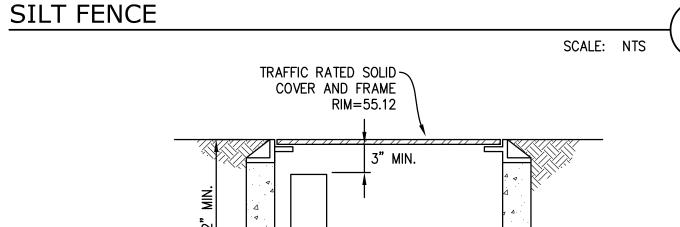
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NOTE: FILTER FABRIC FENCE SHALL BE INSTALLED ALONG CONTOUR WHENEVER POSSIBLE.

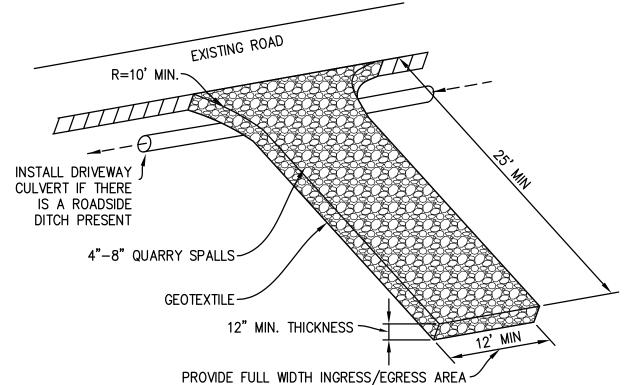
MAINTENANCE STANDARDS

- ANY DAMAGE SHALL BE REPAIRED IMMEDIATELY.
- 2. IF CONCENTRATED FLOWS ARE EVIDENT UPHILL OF THE FENCE, THEY MUST BE INTERCEPTED AND CONVEYED TO A SEDIMENT TRAP OR POND.
- 3. IT IS IMPORTANT TO CHECK THE UPHILL SIDE OF THE FENCE FOR SIGN OF THE FENCE CLOGGING AND ACTING AS A BARRIER TO FLOW AND THEN CAUSING CHANNELIZATION OF FLOWS PARALLEL TO THE FENCE. IF THIS OCCUR, REPLACE THE FENCE AND/OR REMOVE THE TRAPPED SEDIMENT.
- 4. SEDIMENT MUST BE REMOVED WHEN THE SEDIMENT IS 6" HIGH.
- 5. IF THE FILTER FABRIC HAS DETERIORATED DUE TO ULTRAVIOLET BREAKDOWN, IT SHALL BE REPLACED.



4" PVC OUTLET I.E.=53.62 W REMOVABLE 4"ø TYPE 40 CB-

OIL SEPARATOR CB



MAINTENANCE STANDARDS

- QUARRY SPALLS (OR HOG FUEL) SHALL BE ADDED IF THE PAD IS NO LONGER IN ACCORDANCE WITH THE SPECIFICATIONS.
- 2. IF THE ENTRANCE IS NOT PREVENTING SEDIMENT FROM BEING TRACKED ONTO PAVEMENT, THEN ALTERNATIVE MEASURES TO KEEP THE STREETS FREE OF SEDIMENT SHALL BE USED. THIS MAY INCLUDE STREET SWEEPING, AN INCREASE IN THE DIMENSIONS OF THE ENTRANCE, OR THE INSTALLATION OF A WHEEL WASH. IF WASHING IS USED, IT SHALL BE DONE ON AN AREA COVERED WITH CRUSHED ROCK, AND WASH WATER SHALL DRAIN TO A SEDIMENT TRAP OR POND.
- 3. ANY SEDIMENT THAT IS TRACKED ONTO PAVEMENT SHALL BE REMOVED IMMEDIATELY BY SWEEPING. THE SEDIMENT COLLECTED BY SWEEPING SHALL BE REMOVED OR STABILIZED ON—SITE. THE PAVEMENT SHALL NOT BE CLEANED BY WASHING DOWN THE STREET, EXCEPT WHEN SWEEPING IS INEFFECTIVE AND THERE IS A THREAT TO PUBLIC SAFETY. IF IT IS NECESSARY TO WASH THE STREET, THE CONSTRUCTION OF A SMALL SUMP SHALL BE CONSIDERED. THE SEDIMENT WOULD THEN BE WASHED INTO THE SUMP.

 4. ANY ROCK SPALLS THAT ARE LOOSENED FROM THE PAD AND END UP ON THE ROADWAY SHALL BE
- REMOVED IMMEDIATELY.
- IF VEHICLES ARE ENTERING OR EXITING THE SITE AT POINTS OTHER THAN THE CONSTRUCTION ENTRANCE(S), FENCING (SECTION 5.4.1) SHALL BE INSTALLED TO CONTROL TRAFFIC.

ROCK CONSTRUCTION ENTRANCE

SCALE: NTS

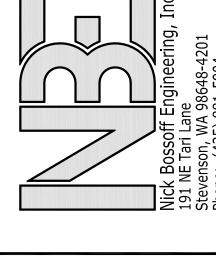
-OVERFLOW BYPASS FOR PEAK STORM GEOTEXTILE ~ VOLUMES **FABRIC** -SEDIMENT ACCUMULATION

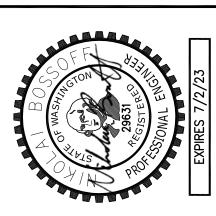
- 1. INSERT SHALL BE INSTALLED PRIOR TO CLEARING AND GRADING ACTIVITY, OR UPON PLACEMENT OF A NEW CATCH BASIN.
- 2. SEDIMENT SHALL BE REMOVED FROM THE UNIT WHEN IT BECOMES HALF FULL.
- 3. SEDIMENT REMOVAL SHALL BE ACCOMPLISHED BY REMOVING THE INSERT, EMPTYING, AND RE-INSERTING IT INTO THE CATCH BASIN.

CB INSERT

ADAPTOR SKIRT~ TRIM TO WITHIN 3" - 5" OF GRATE

SCALE: NTS





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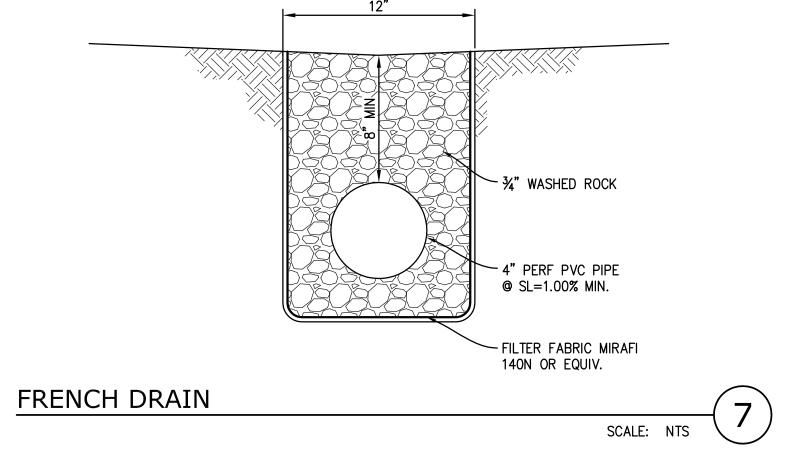
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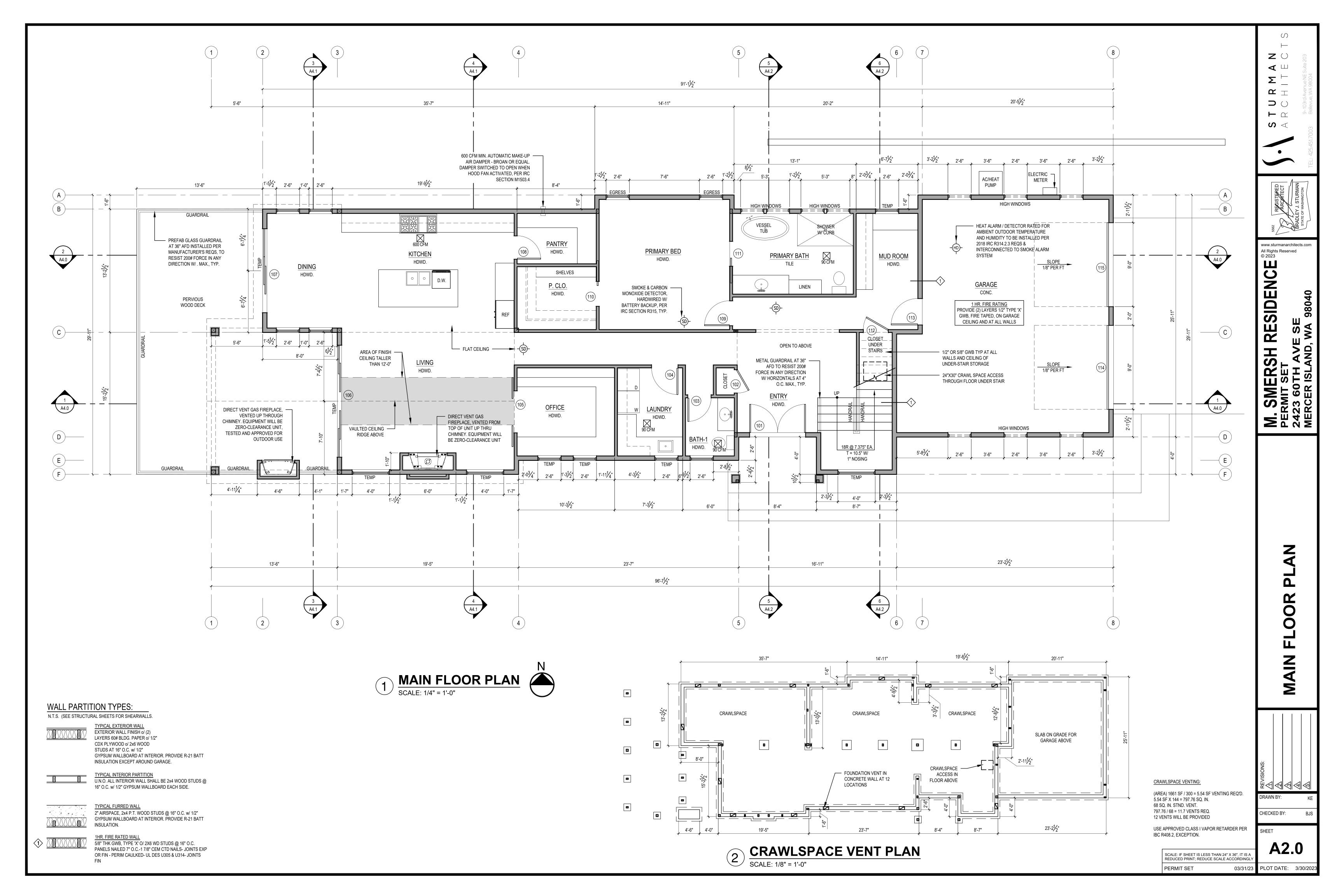
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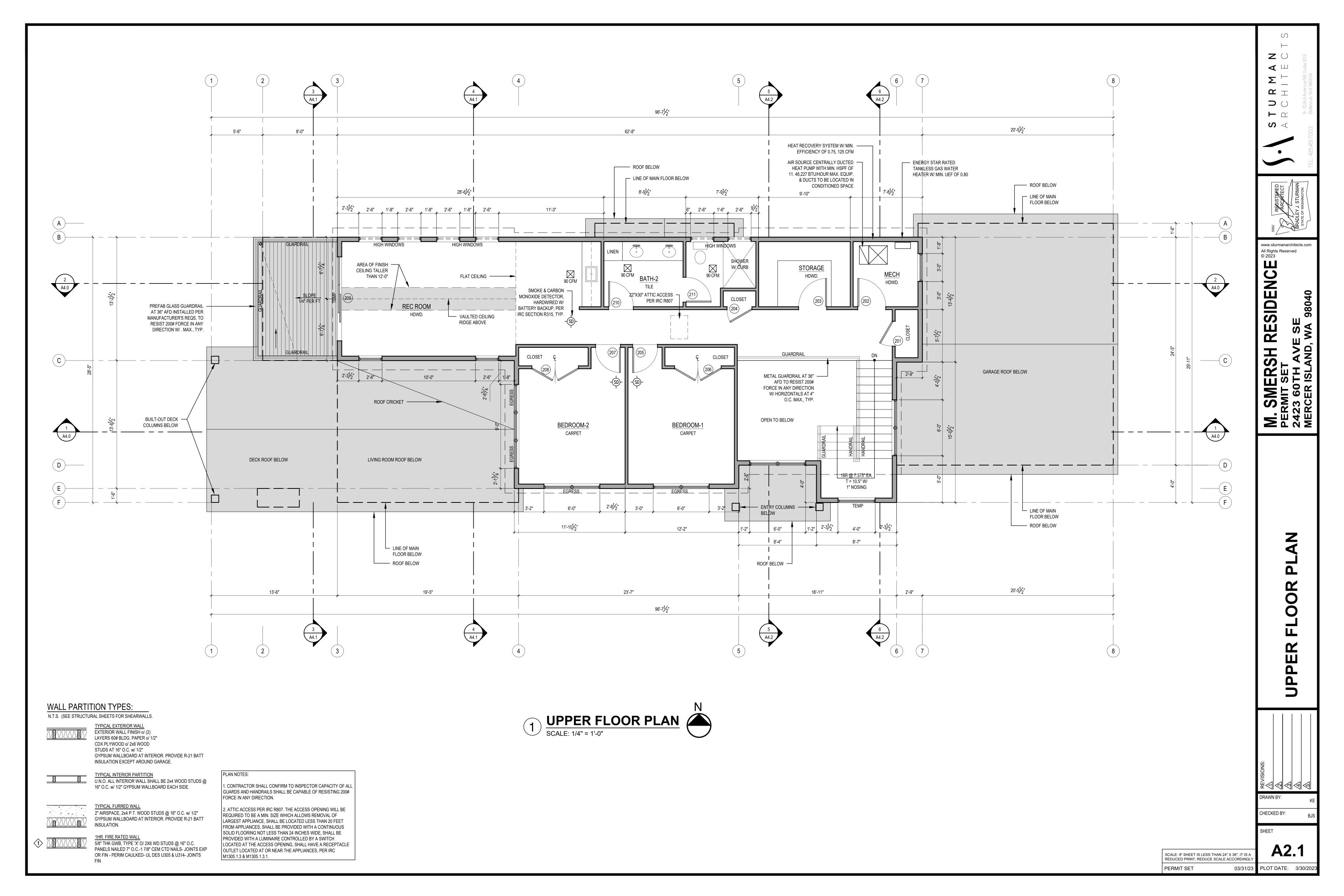
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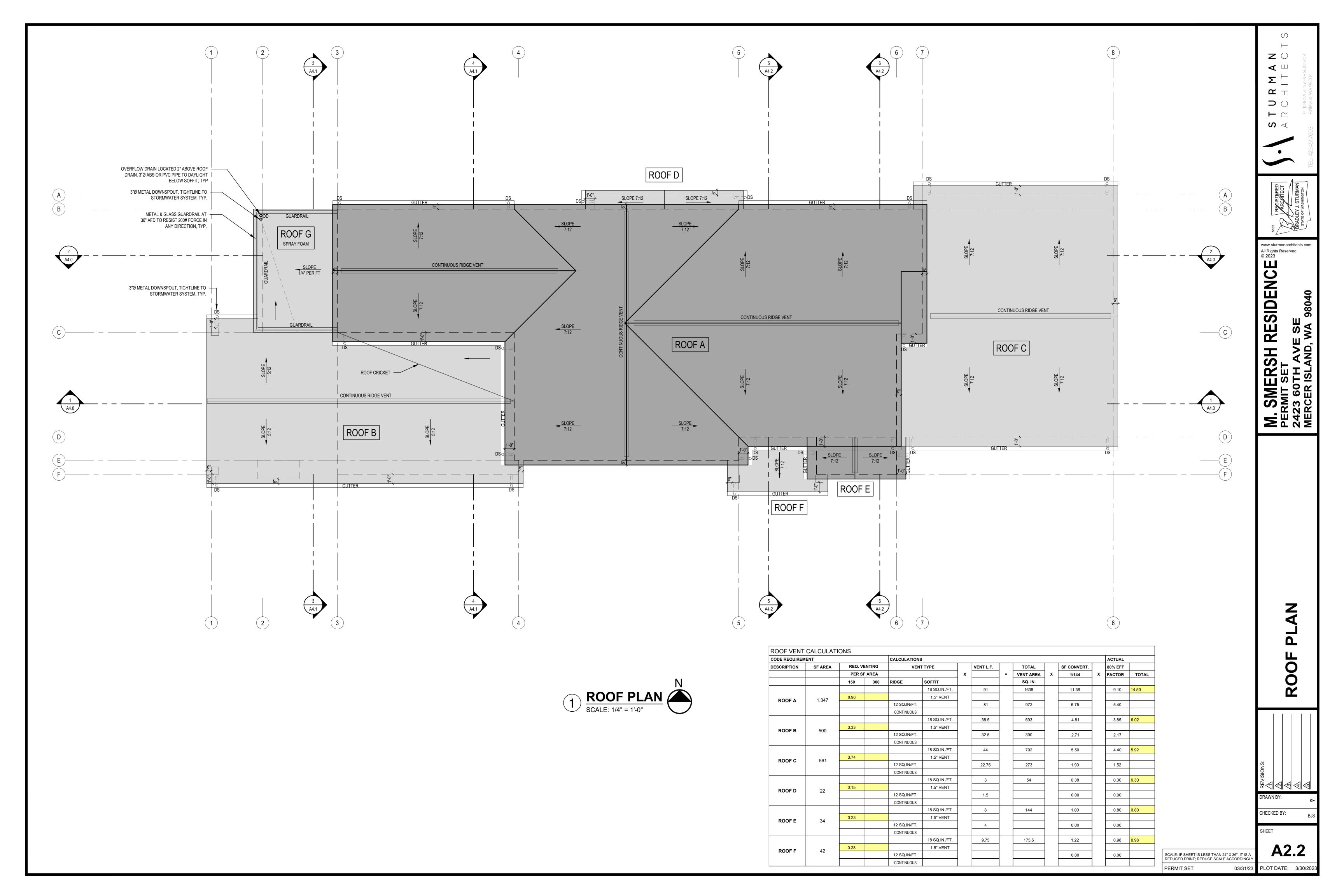
MERCER ISLAND

DETAILS



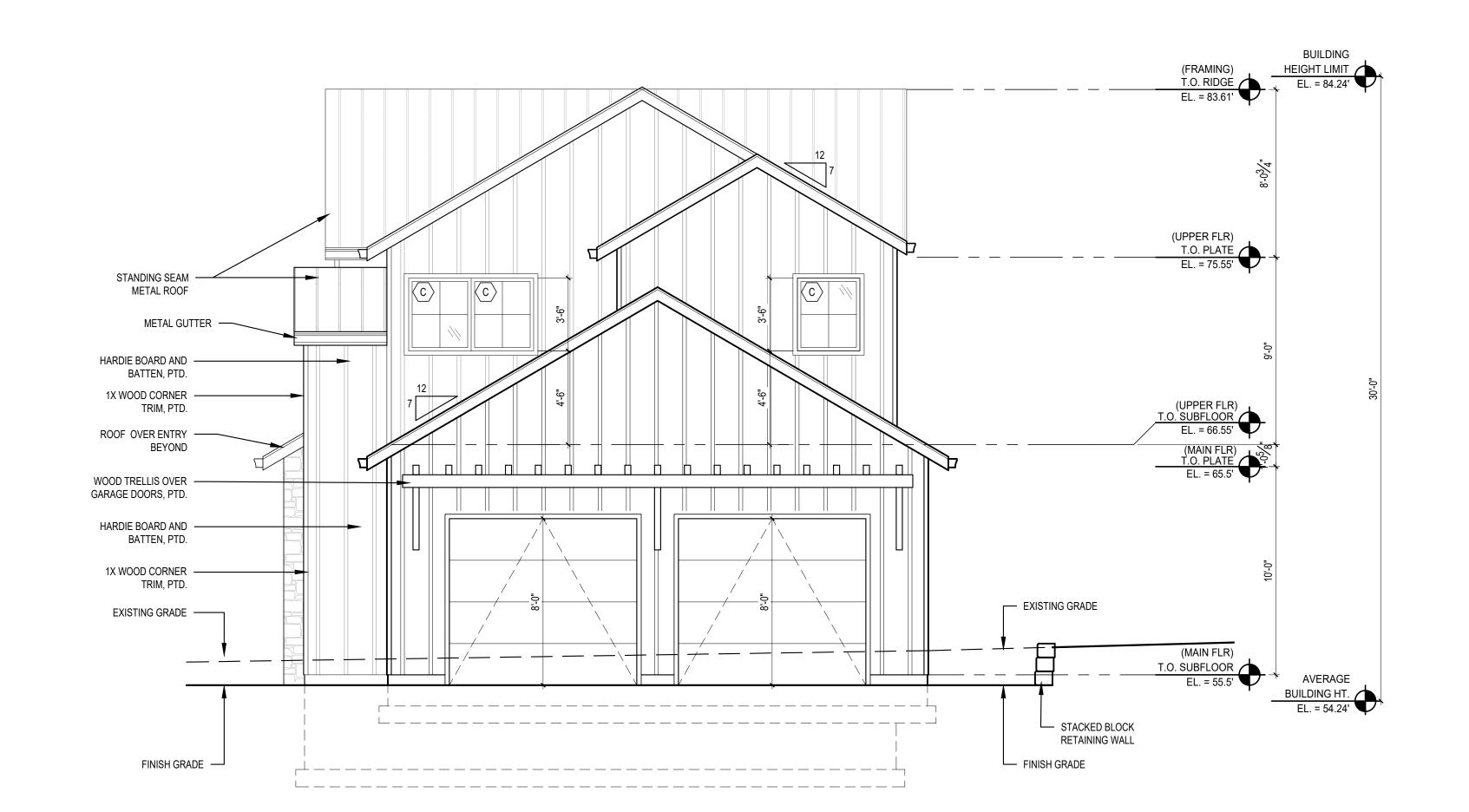






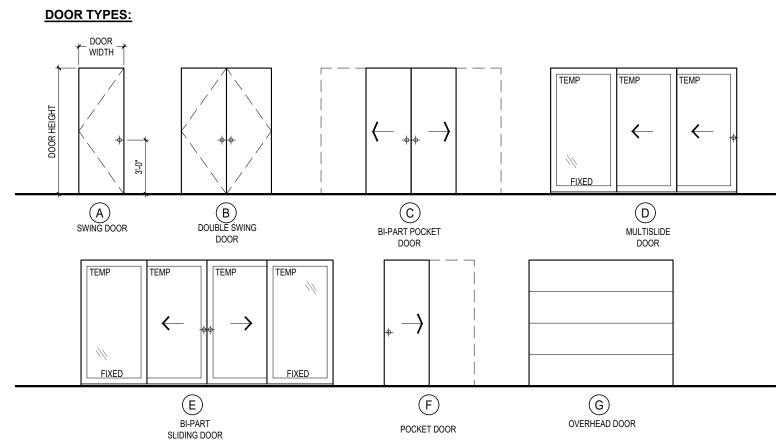






VIND	OW SCHEDU	JLE - S	mersh l	Reside	ence				
TAG	DESCRIPTION	R.O.	SIZE	TEMP.	QTY.	AREA (SF)	U-VAL (MIN.)	GLAZING	REMARKS & NOTES
		WIDTH	HEIGHT						
Α	CASEMENT	2'-6"	5'-0"	TEMP	5		0.28	LOW E / CLEAR	SOME TEMPERED
В	CASEMENT	3'-0"	5'-0"		4		0.28	LOW E / CLEAR	EGRESS
С	FIXED	3'-0"	3'-6"		5		0.28	LOW E / CLEAR	
D	FIXED	4'-0"	10'-0"	TEMP	1		0.28	LOW E / CLEAR	TEMPERED GLASS
Е	FIXED	2'-6"	5'-0"		4		0.28	LOW E / CLEAR	
F	FIXED	4'-0"	7'-0"	TEMP	2		0.28	LOW E / CLEAR	TEMPERED GLASS
G	CASEMENT	2'-6"	4'-6"	TEMP	4		0.28	LOW E / CLEAR	SOME TEMPERED
Н	FIXED	2'-6"	2'-6"		10		0.28	LOW E / CLEAR	
I	FIXED	4'-4"	3'-6"		1		0.28	LOW E / CLEAR	TRANSOM, SLANTED HEAD
I1	FIXED	4'-4"	3'-6"		1		0.28	LOW E / CLEAR	TRANSOM, SLANTED HEAD, REVERSE
J	CASEMENT	3'-0"	4'-6"		3		0.28	LOW E / CLEAR	EGRESS
K	FIXED	5'-9"	3'-11"		1		0.28	LOW E / CLEAR	TRANSOM, SLANTED HEAD
K1	FIXED	5'-9"	3'-11"		1		0.28	LOW E / CLEAR	TRANSOM, SLANTED HEAD, REVERSE
L	FIXED	2'-6"	2'-6"		3		0.28	LOW E / CLEAR	SHOWER WINDOWS
М	AWNING	2'-6"	2'-6"		3		0.28	LOW E / CLEAR	
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DOOR NO.	LOCATION	SIZE WIDTH	SIZE HEIGHT	DOOR TYPE	TEMP.	DOOR THK.	U-VAL (MIN.)	REMARKS
IAIN FLO	DOR	1 *********	TILIOTTI		102.00		,	
101	ENTRY	PR 3'-0"	8'-0"		Y	1-3/4"	0.28	TEMPERED GLASS
102	ENTRY CLOSET	2'-6"	8'-0"			1-3/4"		
103	BATH-1	2'-6"	8'-0"			1-3/4"		
104	LAUNDRY	2'-10"	8'-0"			1-3/4"		SOUND GASKET
105	OFFICE	PR 2'-3"	8'-0"			1-3/4"		BI-PART POCKET
106	LIVING	12'-0"	8'-0"		Υ	1-3/4"	0.28	TEMP, BI-PART SLIDER
107	DINING	9'-0"	8'-0"		Υ	1-3/4"	0.28	TEMPERED GLASS
108	PANTRY	2'-6"	8'-0"			1-3/4"		
109	PRIMARY BEDROOM	2'-8"	8'-0"			1-3/4"		
110	PRIMARY CLOSET	2'-8"	8'-0"			1-3/4"		
111	PRIMARY BATH	2'-8"	8'-0"			1-3/4"		POCKET
112	UNDERSTAIR CLOSET	2'-8"	8'-0"			1-3/4"		
113	GARAGE	2'-10"	8'-0"			1-3/4"		20 MIN FIRE RATED, GASKET
114	GARAGE	9'-0"	8'-0"			1-3/4"		OVERHEAD DOOR
115	GARAGE	9'-0"	8'-0"			1-3/4"		OVERHEAD DOOR
IPPER F	LOOR							
201	HALL CLOSET	2'-8"	8'-0"			1-3/4"		
202	MECHANICAL	3'-0"	8'-0"			1-3/4"		SOUND GASKET
203	STORAGE	3'-0"	8'-0"			1-3/4"		
204	HALL CLOSET	2'-8"	8'-0"			1-3/4"		
205	BEDROOM-1	2'-8"	8'-0"			1-3/4"		
206	BED-1 CLOSET	PR 2'-6"	8'-0"			1-3/4"		
207	BEDROOM-2	2'-8"	8'-0"			1-3/4"		
208	BED-2 CLOSET	PR 2'-6"	8'-0"			1-3/4"		
209	REC ROOM	9'-0"	8'-0"		Y	1-3/4"	0.28	TEMPERED GLASS
210	BATH-2	2'-8"	8'-0"			1-3/4"		
211	BATH-2	2'-8"	8'-0"			1-3/4"		



1.) CONTRACTOR TO VERIFY ALL GLAZING SIZING, AND DOOR DIMENSIONS IN FIELD PRIOR TO ROUGH FRAMING & ORDERING OF GLAZING/WINDOW/DOOR MATERIALS. REVIEW SIZES AND

4.) GLAZING INDOORS AND/OR WITHIN 24" OF A DOOR TO BE TEMPERED. SEE EXTERIOR ELEVATION FOR TEMP. GLASS LOCATION & EGRESS WINDOWS.

5.) 2018 WSEC & VIAQ RESIDENTIAL PRESCRIPTIVE OPTION 3 ADOPTED. GLAZING AREA INDICATED UNLIMITED. SEE ENERGY NOTE AT A1.0 SHEET FOR DETAILS.

6.) ALL WINDOWS AND DOORS WITHOUT A BUG ARE EXISTING TO REMAIN.

PERMIT SET

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PLOT DATE: 3/30/202

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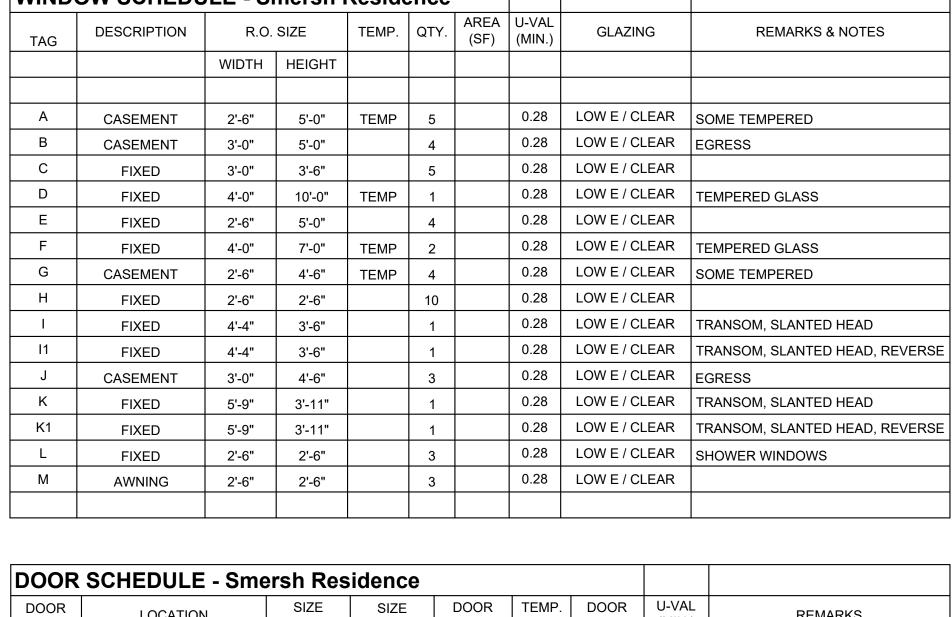
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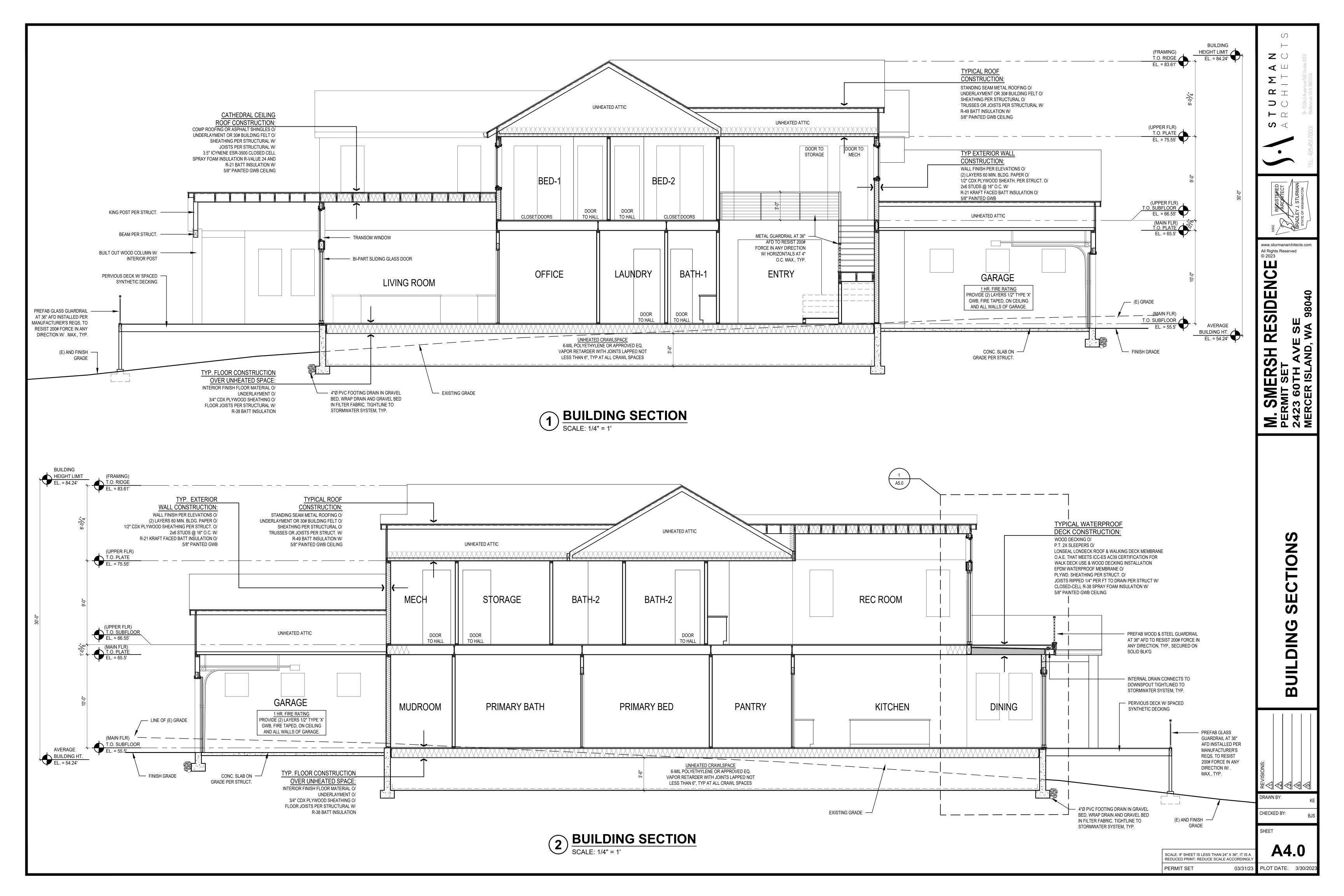


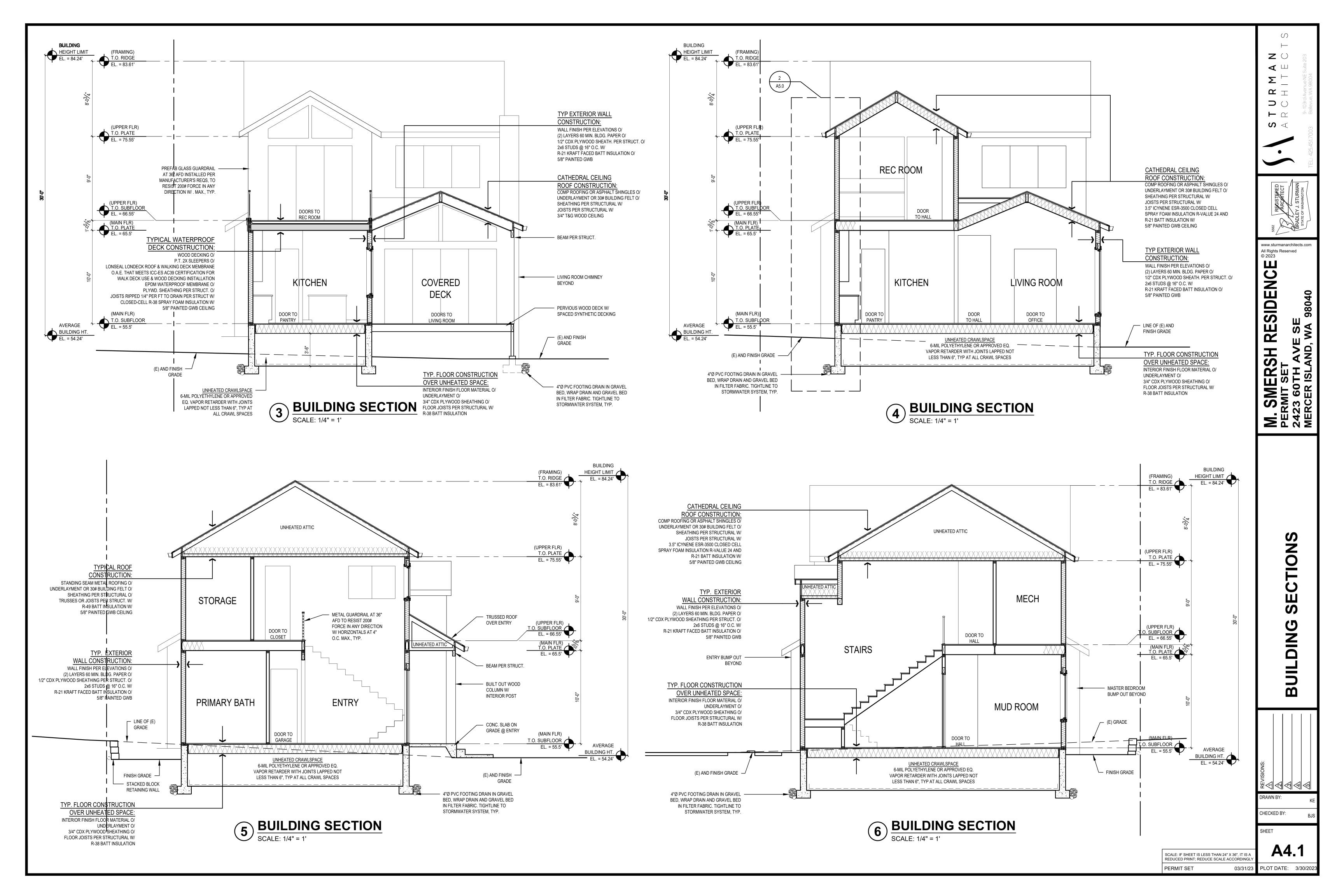
WINDOW & DOOR SCHEDULE NOTES:

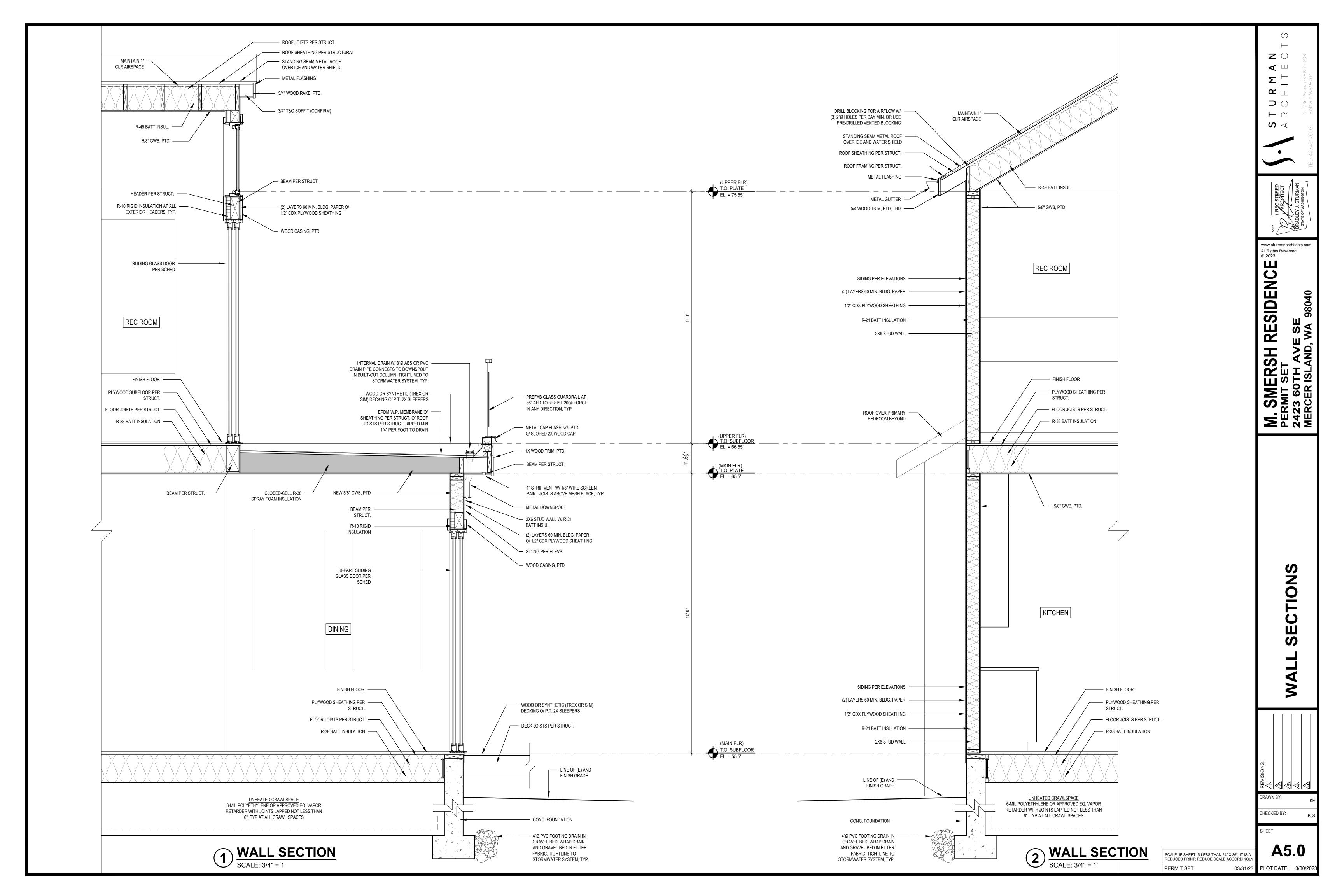
ANY DISCREPANCIES W/ ARCHITECT.

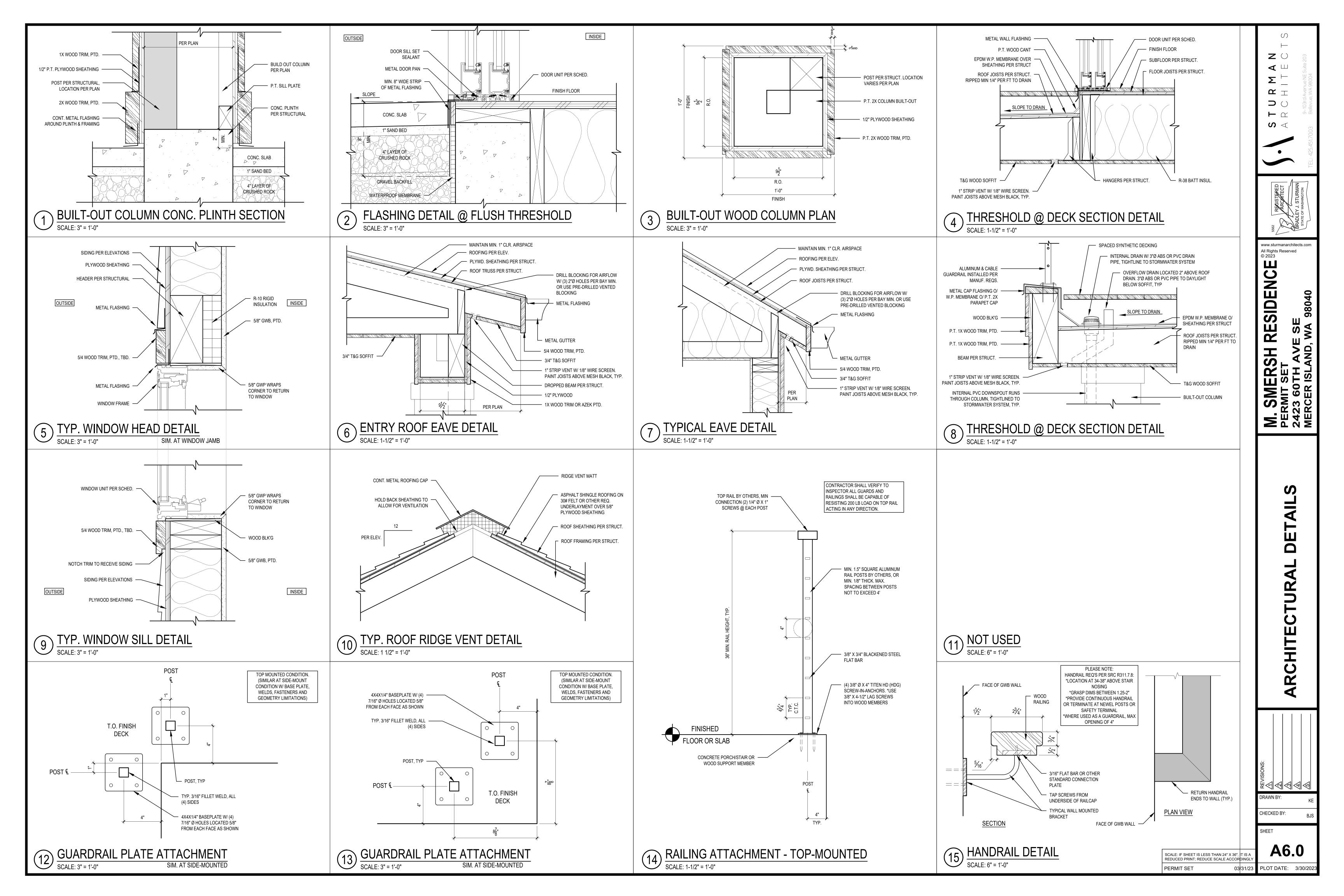
2.) ALL GLAZING TO BE "LOW E", INSULATED GLASS UNLESS NOTED OTHERWISE.

3.) ALL OPERABLE WINDOWS TO HAVE SCREENS.









GENERAL NOTES

1.0 GENERAL

- 1.1 Construction shall conform to the 2018 INTERNATIONAL RESIDENTIAL CODE and all other requirements of authorities having jurisdiction.
- 1.2 These drawings are the property of O.G. Engineering, PLLC ("Engineer"). These drawings and the information contained herein shall not be used for completion of or revisions to this project by others, extensions of this project or any other project without Engineer's express written permission.
- 1.3 Refer to Architectural Plans for all dimensions and elevations not shown. Do not scale drawings. The contractor shall verify all pertinent dimensions and existing conditions prior to beginning construction. Conflicts, differences in information, and omissions in drawings shall be brought to the attention of the Engineer for resolution prior to construction. Changes from the drawings shall be made only with the prior approval of the Engineer. All work is subject to review and approval by the local building department. All work shall conform to all permit and building department requirements. All details shall be considered typical at similar conditions. Details shall be used where applicable, unless otherwise noted. Details intend to show concepts that may not exactly match specific site conditions. All work shown on these drawings is new unless noted as existing.
- 1.4 The contractor shall be solely responsible for jobsite and construction safety and compliance with all current safety regulations. Jobsite visits performed by the Engineer do not include a review of the adequacy of the contractor's safety measures. The Engineer has no authority to exercise any control over any construction contractor or their employees in connection with their work or any health or safety precautions. Only the final, permanent structure is shown on these drawings. The contractor shall be solely responsible for the means and methods of construction, including but not limited to construction sequencing and providing all necessary shoring, bracing and other temporary supports during construction. The contractor shall be solely responsible for obtaining all necessary independent engineering reviews of all temporary conditions and support systems during construction.
- 1.5 Utility information is not shown on these drawings. The contractor shall be solely responsible for locating and protecting utilities prior to and during construction. The contractor shall be solely responsible for all damage to utilities resulting from their work, and all damage to utilities shall be repaired solely at the contractor's expense.
- 1.6 All waterproofing and drainage information shown on these drawings is for illustrative purposes only. Waterproofing and drainage are the design responsibility of others.

2.0 DESIGN BASIS - BUILDING STRUCTURES

2.1 Vertical Loads (psf)	Dead	Live	Snow
Truss Roof	19*		25
Stick—Framed Roof	18*		25
Roof Deck	18	60	
Upper Floor	14	40	
Main Floor	11	40	
*Includes 4psf for solar-re	eady zones	3	

2.2 Seismic Design Data (per the 2018 IBC)

Risk Category: II Importance Factor: le=1.0

Site Coordinates: 47.5818°N, 122.2136°W Mapped Spectral Response Acceleration: Ss=1.40, S1=0.49

Site Class: D (per Geotech Engr)

Spectral Response Coefficients: Sds=0.93 Seismic Design Category: D

Main Seismic Force—Resisting System: Wood Structural Panel

Shear Walls

Response Modification Factor: R=6.5

Seismic Response Coefficient: Cs=0.14 Redundancy Factor: $\rho=1.3$

Over-strength Factor: Ω =2.5

Analysis Procedure Used: Equivalent Lateral Force Procedure

2.3 Wind Design Data (per the 2018 IBC)

Risk Category: II

Basic Wind Speed: 98 mph

Exposure Category: C Topographic Factor: 1.00 (Per Mercer Island Wind Load Map)

3.0 INSPECTIONS

The construction work shall be inspected as required by the SRC Section R106. The contractor is solely responsible for understanding the requirements of and coordinating all inspections, observations and testing and ensuring that all work is performed to the satisfaction of the inspector.

4.0 FOUNDATIONS

4.1 New foundations have been designed in accordance with recommendations in the Geotechnical Report. The design basis is as follows:

* Allowable Vertical Bearing Pressures: Dead + Live Dead + Live + Short Term 3325 psf * Sliding Resistance: 300 pcf Passive Pressure Sliding Friction Coefficient

4.2 All site preparation, grading, earthwork and site drainage, including but not limited to sub-grade preparation, foundation and retaining wall excavations, structural fill specifications, compaction requirements, and site drainage installation, shall be performed in accordance with the Geotechnical Report prepared by the Geotechnical Engineer, Geotech Consultants, Inc., dated March 3rd, 2022. The Geotechnical Report is part of the construction documents and a copy may be obtained from the Geotechnical Engineer's office. The contractor shall notify Geotech Consultants, Inc. (425-747-5618) in advance of any earthwork operations and Geotech Consultants, Inc. should be present to observe and test, as necessary, the earthwork and foundation installation phases of the project.

5.0 MATERIALS

5.1 Wood:

5.1.1 All 2x & 3x sawn lumber shall be Hem Fir arade number 2. and all 4x and larger lumber shall be Doug Fir grade number 1, U.O.N. Mudsills and all sawn lumber in contact with concrete, masonry, around, exposed to weather or moisture, shall be P.T. Preservative retention levels in P.T. wood shall meet the requirements of the applicable use category in accordance with AWPA U1-16, and shall not exceed those required to comply with AWPA Use Category UC4A. Do not use wood treated with ACZA. Field-cut ends, notches and drilled holes of P.T. wood shall be treated in the field in accordance with AWPA M4. P.T. is not required at naturally decay-resistant (i.e. redwood, cedar etc.)

5.1.2 Engineered Wood Framing Members and I-Joists shall be TrusJoist® or approved equal. 'PSL' denotes Parallam 2.2E for beams and 1.8E for posts. 'LSL' denotes Timberstrand 1.55E for members with depth equal to or greater than $9\frac{1}{2}$, and 1.3E for members with depth less than $9\frac{1}{2}$ ". 'LVL' denotes Microllam 2.0E. 'TJI' denotes TJI I-joists.

5.1.3 Glulam framing members shall be DF/DF, stress class 24F-1.8E, combination symbol 24F-V8, U.O.N.

5.1.4 All wood framing members shall have 19% maximum moisture content at time of installation.

5.2 Concrete:

Hardrock, normal-weight concrete with a minimum 28-day compressive strength of 3,000 psi for concrete exposed to weather and 2,500psi for concrete not exposed to weather. Slump range shall be 3-5 inches. Maximum aggregate size shall be 1". Maximum water/cement ratio shall be 0.5. Concrete exposed to weather shall be air—entrained with total air content between 5%-7% of total concrete volume.

5.3 Reinforcing Steel Bars:

ASTM A615, Grade 60

sawn lumber members.

5.4 Post-Installed Dowels & Anchors into Existing Concrete & CMU

Epoxy: Simpson SET-3G (Installed & inspected per ICC No. ESR-4057)

5.5 Bolts and Threaded Rods:

5.5.1 Threaded Rod: ASTM F1554 Grade 36

5.5.2 Sill Anchor Bolts: ASTM A307

Bent bar "J" anchor bolts shall have a hook with a 90-dearee bend with an inside diameter of three bolt diameters, plus an extension of one and one half bolt diameters at the free end.

5.5.3 Bolts in Timber Connections: ASTM A307

5.5.4 Bolts in Steel Connections: ASTM A325-N (High-Strength)

5.6 Structural Steel:

Wide Flange (W): A992 (Fy = 50 ksi) Rectangular Tube (HSS): A500 Gr. B (Fy = 46 ksi) A36 (Fy = 36 ksi)Plate and Bar:

6.0 CONCRETE CONSTRUCTION

- 6.1 Concrete elements shall be constructed in single continuous pours. without construction joints, unless otherwise approved by the Engineer, Reinforcement shall be the longest lengths practical Splices in rebar are not allowed in footings or walls less than 20 feet long. Lap splices shall be staggered at least 2 ft. in adjacent bars. Where reinforcement or anchor edge distances are noted on the drawings as "clear", the distance shall be taken from the face of reinforcement or anchor to edge of concrete. Cast—in—place reinforcement and anchor bolts shall be installed prior to concrete placement and shall not be "wet-set" into freshly poured concrete.
- 6.2 Reinforcement installation details, including rebar bends, hooks, splices and development lengths shall be in accordance with the requirements of IRC Section R608.5.4. U.O.N. Concrete materials. forms, mixing and delivery shall be in accordance with the requirements of the IRC Section R404.1.3.3.
- 6.3 Concrete Coverage over Reinforcing Steel

Unless otherwise noted, maintain the minimum concrete cover to face of reinforcement or anchors as follows:

1) 3" Where concrete is cast against and permanently exposed to earth except slab on grade.

2) 2" Where concrete is exposed to earth but formed, or exposed to weather.

3) $1\frac{1}{2}$ Where concrete is not exposed to earth or weather.

6.4 Slabs on Grade

6.4.1 Crack Control Joints

Cut crack control joints in top of slab @10'-0"o.c. (max.) each way. Joint depth shall be $\frac{1}{4}$ of the slab depth or 1", whichever is greater. Joints shall be conventional saw-cut within 4 to 12 hrs of concrete placement, or early-entry saw-cut within 1 to 4 hrs of concrete placement. Jointed panels shall be rectangular, as square as possible, with a max length—to—width ratio of $1\frac{1}{2}$:1.

6.4.2 Slab Sub-Base

Slab sub-base shall be $\frac{5}{8}$ " to $\frac{3}{4}$ " clean, crushed drain rock, compacted to a firm and unyielding condition.

7.0 WOOD CONSTRUCTION

7.1 General Framing

Connections not specified on these drawings shall conform to the IRC fastening schedule, refer to Table R602.3(1). Depth of all posts in walls shall match stud depth, U.O.N. Block floor joist space solid under posts and cripple studs supporting headers and continue support to foundation. Face nail all plies of multi-ply studs with 10d@6"o.c. Obtain approval from engineer prior to ripping or creating notches or holes in framing members. U.O.N. Install double joists below all interior walls parallel to floor joists and solid blocking below all interior walls perpendicular to floor ioists. U.O.N. All beams shall be continuous across supports unless explicitly shown as multiple pieces. Install full depth blocking between framing members over supports, unless otherwise noted. Intall 2x4 blkg btwn adjacent joists/rafters/ trusses @24"o.c. over interior partitions.

7.2 Engineered Wood Framing

See TrusJoist "Installation Guide for Floor and Roof Framing" (TJ-9001) for allowable holes in engineered wood beams. Grade stamp info <u>must be maintained</u> on ripped engineered wood members; refer to TrusJoist Technical Bulletin TB-305 for requirements pertaining to re-sawn engineered wood.

7.3 Fasteners

Nails specified on these drawings are common nails, U.O.N. Fasteners in contact with P.T. wood, exposed to weather or in contact with ground shall be hot-dipped galvanized per SRC Section 317.3, or shall have equivalent corrosion resistance. Dissimilar metals & coatings shall not be in contact. Bolt holes shall be a minimum of $\frac{1}{32}$ " to a maximum of $\frac{1}{16}$ " larger than the bolt diameter. Bolts shall not be forcibly driven, and shall be tightened to the snug—tight condition. Install standard cut washers under all bolt heads and nuts bearing against wood.

7.4 Connectors

Connectors specified on these drawings are manufactured by the SIMPSON STRONG-TIE® Company. Refer to latest catalog for information not specifically noted herein. Connectors in contact with P.T. wood, exposed to weather or in contact with ground shall be ZMAX or HDG galvanized. All connectors shall receive the maximum number of fasteners, U.O.N. Dissimilar metals & coatings shall not be in contact. Shim gaps in connectors for different framing sizes with plywood as required. Non-field-adjustable hangers specified as sloped or skewed shall be manufactured sloped or skewed.

7.5 Wood Structural Panels

WSPs shall bear the APA trademark and shall meet the requirements of the latest edition of USDOC PS1 or PS2. Use 10d common wire nails to fasten panels with $1\frac{1}{2}$ minimum penetration into framing at all panel edge and field nailing, U.O.N. Nails shall be located at least $\frac{3}{8}$ " from panel ends and edges. Stagger nails at adjoining panel edges. Drive nail heads flush with panel surface. Maintain $\frac{1}{8}$ " gap between all adjoining panel edges. Center interior panel joints on framing members or blocking. Provide $\frac{1}{2}$ " space between untreated panel and concrete or masonry. Minimum panel dimension shall be 2'-0". Panel storage and handling during transport and construction shall be in accordance with APA recommendations and shall protect the panels from prolonged exposure to moisture from rain, snow, ground or other sources. WSPs permanently exposed to weather shall be exterior grade.

7.6 Shear Walls and Exterior Wall Sheathing

7.6.1 Shear walls are noted on the plans. Shear walls shall be sheathed with $\frac{1}{2}$ " APA RATED SHEATHING, EXPOSURE 1 WSPs with a span rating of $\frac{32}{16}$, U.O.N. Panels shall not be less than 4'-0" x8'-0", except at boundaries and changes in framing. Panels shall be laid with strength axis vertical. Install 2x blkg under all unsupported panel edges; all panel edges shall be supported by and fastened to min. 2x common studs or blocking, U.O.N. on shear wall schedule. Edge nail panels to posts within shear walls. Install double stud or min. 4x post at the ends of all shear walls. Provide solid blocking under double studs & posts between floors and continue support to foundation. See shear wall schedule for more information.

7.6.2 WSP Wall Nailing, U.O.N.:

Panel Edge Nailing: 10d@6"o.c. maximum. Intermediate (Field) Nailing: 10d@12"o.c. maximum.

7.6.3 All new exterior walls not called out as shear walls shall be sheathed on their exterior face with $\frac{1}{2}$ " APA RATED SHEATHING, EXPOSURE 1 WSPs with a span rating of $\frac{32}{16}$ and nailing per note 7.6.2., U.O.N. All other fasteners & requirements shall conform to the shear wall schedule for wall type (1).

7.7 Holdowns and Tiedown Straps

Holdowns and tiedown straps shall be attached to double studs or min. 4x posts, U.O.N. See latest Simpson Catalog for additional requirements not noted herein. See holdown schedule for anchor bolt sizes and additional specifications. Refer to note 7.1 for nailing and framing requirements at holdown/tiedown posts. Install solid post at shear wall corners or intersections where holdowns/tiedowns occur. All holdowns/tiedowns shall have the maximum number of fasteners.

7.8 Sill Anchor Bolts

There shall be a minimum of two sill anchor bolts per piece with one bolt located not more than 12" or less than $4\frac{1}{2}$ " from each end of each piece. Holes in sills for bolts shall not be oversized. Sill anchor bolts shall be $\frac{5}{8}$ "\varphi\$ with 7" min. embed. into concrete. Sill anchor bolts into existing concrete shall be all—thread rod, drill and epoxy. See shear wall schedule for spacing of sill anchor bolts in shear walls. Maximum sill anchor bolt spacing at non-shear-walls shall be 6'-0"o.c. at interior walls and 4'-0"o.c. at exterior walls. All sill anchor bolts at shear walls and mudsills shall be installed with 0.229"x3"x3" steel plate washers. Edge of sill anchor bolt plate washers shall be located $\frac{1}{2}$ max. from inside face of wall sheathing or rim joist where occurs.

7.9 Floor and Roof Sheathing

7.9.1 Wood structural panel sheets at floors and roofs shall be laid with strength axis perpendicular to supports and continuous over two or more spans, unless otherwise noted on drawings. Stagger adjacent panels 4'-0"o.c. lengthwise.

7.9.2 Unless otherwise noted, typical roof sheathing shall be unblocked §" APA RATED SHEATHING, EXPOSURE 1 WSPs with a span rating of $^{40}\!\!/_{20}$. Panels shall be fastened to framing members with 10d nails @6"o.c. at all supported panel edges and 10d nails @12"o.c. intermediate (field) nailing. Install 'PSCL' sheathing clips (one mid-way between each support) at all unsupported panel

7.9.3 Unless otherwise noted, typical floor sheathing shall be unblocked $\frac{3}{4}$ " APA RATED STURD-I-FLOOR EXPOSURE 1

WSPs with a span rating of $^{48}\!\!/_{24}$ and T&G edges. Panels shall be fastened to framing members with 10d nails @6"o.c. at all supported panel edges and 10d nails @12"o.c. field nailing. Glue sheathing to all supports (including blocking) with $\frac{1}{4}$ " minimum beads of approved adhesive meeting APA specification AFG-01.

7.10 Metal-Plate-Connected Wood Trusses

7.10.1 The design, manufacture and installation of trusses shall be in accordance with the requirements of ANSI/TPI 1 and the IRC Section R502.11.

7.10.2 Trusses, structural fascia, their connections to other trusses/fascias, and truss eave blocking are the design responsibility of the supplier, and shall be designed by a civil or structural engineer licensed in the State of Washington ("Truss Designer"). Trusses shall be designed to support the following specific unfactored loads in addition to their self-weight:

<u>Vertical Roof Loads - Top Chord</u>

*Dead: 15 psf (Does not include truss self-weight)

*Snow: 25 psf *Wind: -51 psf (uplift)

<u>Vertical Ceiling Loads — Bottom Chord</u> *Dead: 5 psf (Does not include truss self-weight)

*Live: 10 psf (Does not act concurrently with roof live load)

Lateral Drag Truss Loads Drag trusses shall be designed for a uniform unfactored seismic load acting longitudinally along the entire top chord length, and for a longitudinal reaction acting at the location at which the bottom chord is strapped to the adjacent collector element. The magnitude of the uniform top chord load and concentrated bottom chord load are indicated on plan. The bottom chord strap location is indicated on plan.

7.10.3 Trusses shall not rely on interior walls for support, U.O.N.; trusses shall be designed to span between exterior bearing walls.

7.10.4 Trusses shall be braced to provide lateral stability and prevent rotation in accordance with the SBCA BCSI "Guide to Good Practice for Handling, Installing and Bracing of Metal-Plate-Connected Wood Trusses". Bracing shall be designed and specified by the truss designer.

7.10.5 Trusses and their connections shall not be notched, cut, spliced or otherwise altered or damaged in any way without the prior written consent of both the E.O.R. and truss designer.

7.10.6 Truss design drawings and calculations, prepared by a civil or structural engineer licensed in the State of Washington in accordance with the SRC Section R502.11.4, shall be submitted to the contractor, architect, engineer and local building official for review and acceptance prior to fabrication, and shall be provided with the shipment of trusses to the job site.

7.10.7 Attach top plates of interior, non-bearing partition walls to truss bottom chords with 'STC' clips, leaving a $\frac{1}{4}$ " to $\frac{1}{2}$ " vertical gap between bottom of truss and top of plate. Attach adjacent gypsum board ceiling to top plate with 'DS' clips. Do not fasten gypsum board ceiling to truss bottom chord within 16" of top

ABBREVIATIONS

ADJACENT ALTERNATE ALT. ARCHITECT ARCH. A.T.R. ALL-THREAD ROD

BEAM

B.F. BALLOON-FRAMED BLOCKING BLKG BLW. BELOW

BOTT. воттом C.I.P. CAST-IN-PLACE

C.J. CONSTRUCTION JOINT CL CENTERLINE CLR. CLEAR

ВМ

CONT. CONTINUOUS CSK. COUNTERSINK

DIAMETER DBL. DOUBLE

DF DOUGLAS FIR

DIM DIMENSION D.J. DOUBLE JOIST

D.R. DOUBLE RAFTER E.J. **EXPANSION JOINT**

ELEV. ELEVATION EMBED. EMBEDMENT

ENGINEER

ENGR.

E.N. EDGE NAILING E.O.R. ENGINEER OF RECORD

EQ. EQUAL E/W EACH WAY

(E) **EXISTING** F.J. FLOOR JOIST

FIELD NAILING F.N. FTG FOOTING G.L. GRID LINE

GLB GLULAM BEAM G.C. GENERAL CONTRACTOR

HOT-DIPPED GALVANIZED HDR HEADER

2018 INTERNATIONAL BUILDING CODE®

HEM FIR

INVERTED

IRC 2018 INTERNATIONAL RESIDENTIAL CODE® KILN-DRIED LUMBER

LOCN LOCATION

MAXIMUM MANUFACTURER

MACHINE BOLT

MIN. MINIMUM

NOT SHOWN FOR CLARITY

OVER

ON CENTER o.c. 0/H OPPOSITE HAND

OPNG OPENING

> PL PLATE

NOT TO SCALE

PSF POUNDS PER SQUARE FOOT РΤ PRESSURE-PRESERVATIVE-TREATED

QUAD. QUADRUPLE

REQUIRED

RFT RETROFIT R.R. ROOF RAFTER

R.W. REDWOOD

SEE ARCHITECTURAL DRAWINGS SLAB ON GRADE

S.O.G. SIM. SIMILAR

SQ. SQUARE STD STANDARD

SHEAR WALL SCHEDULE TO BE DETERMINED

TOP & BOTTOM

T&G TONGUE & GROOVE TYP. **TYPICAL**

TRPL. TRIPLE

T.O. TOP OF UNLESS OTHERWISE NOTED

U/S UNDERSIDE

UNDER VERIFY IN FIELD V.I.F.

WESTERN RED CEDAR

W.P. WATERPROOFING WOOD STRUCTURAL PANEL PERMIT SET

ENGINEER OF RECORD

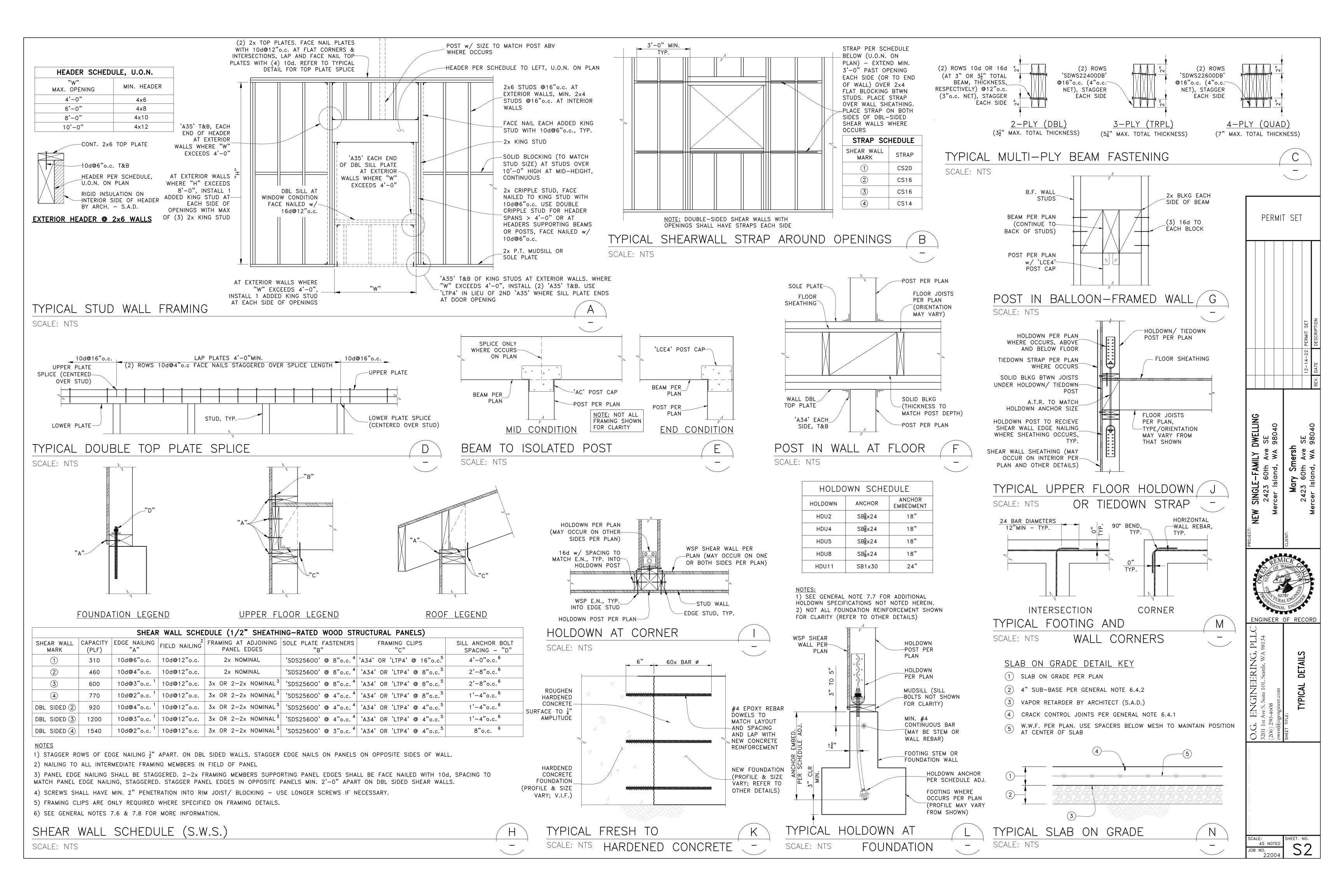
SINGLE—FAN 2423 60th rcer Island,

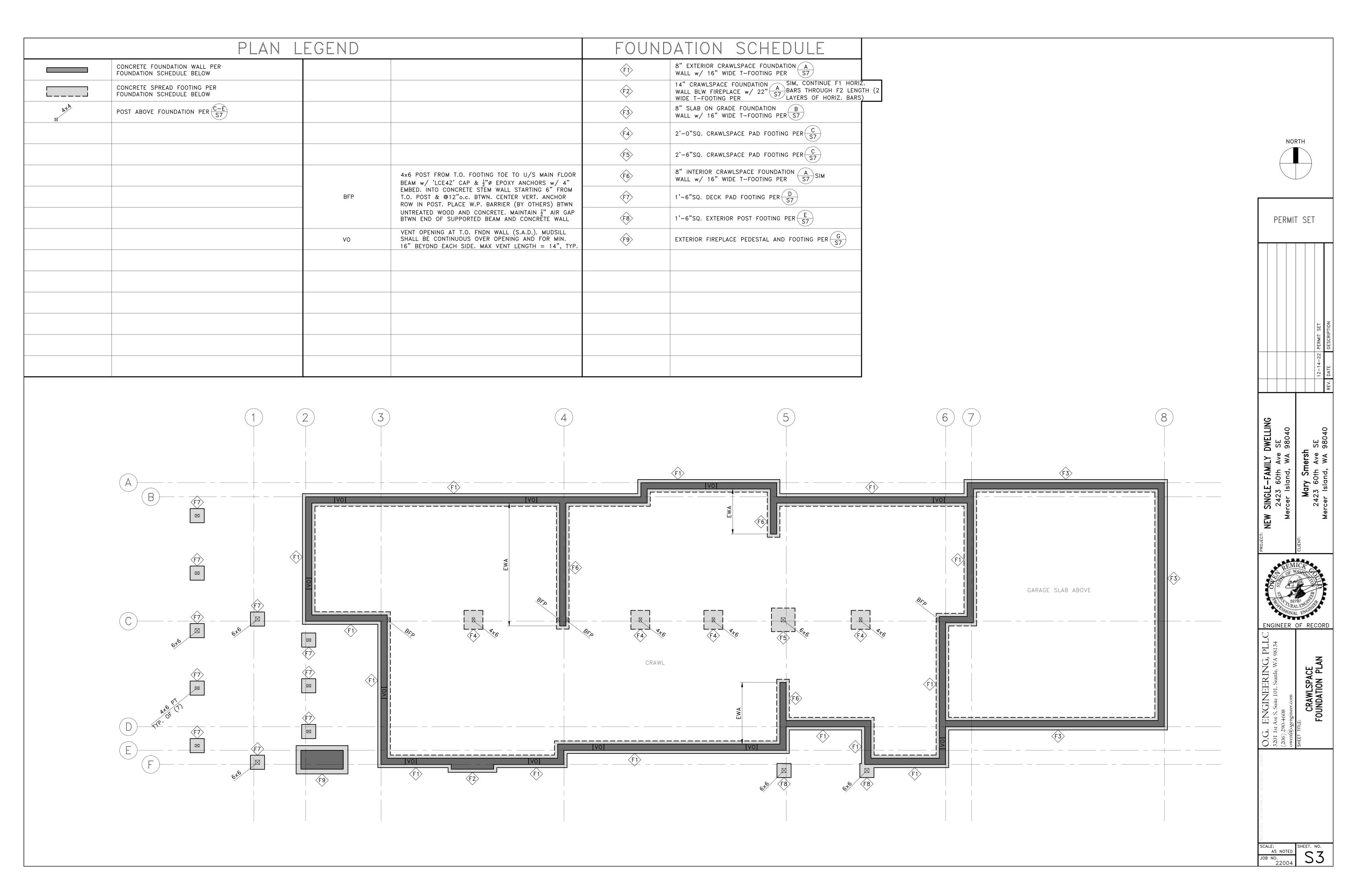
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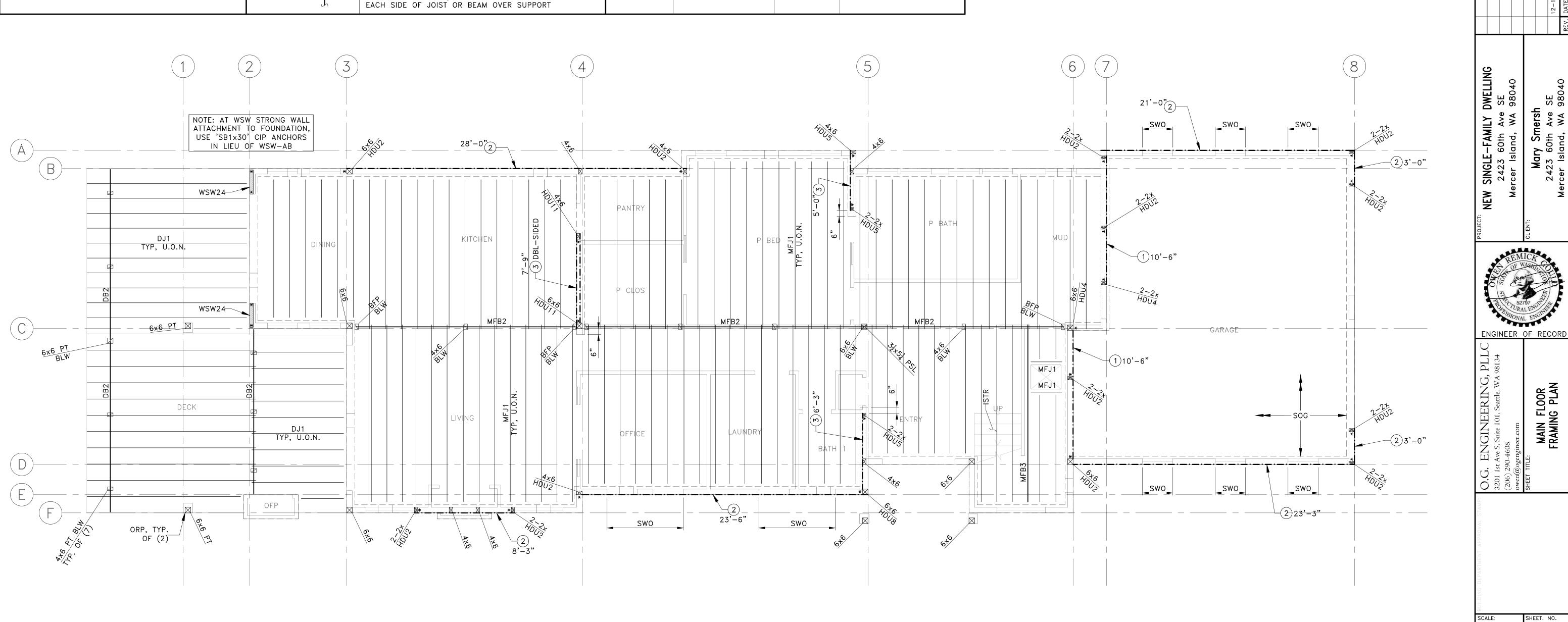
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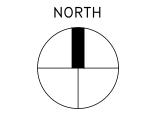
GENERAL



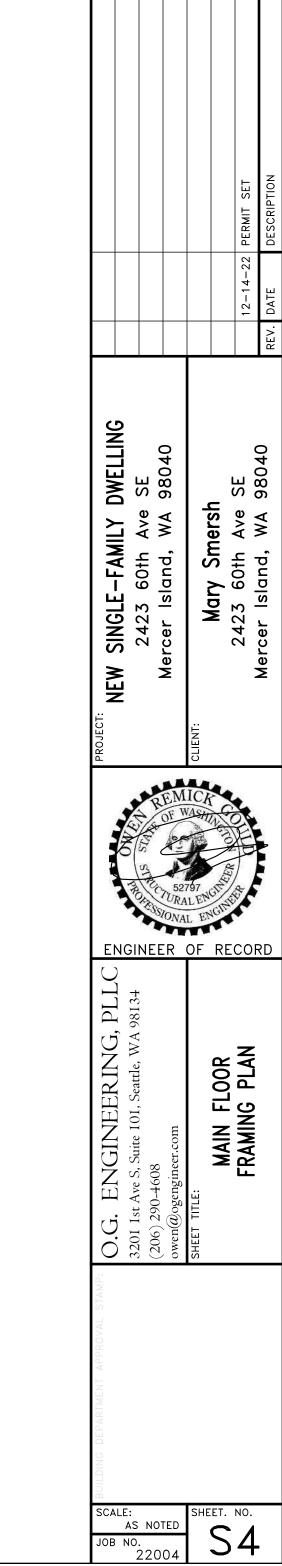


	PLAN L	EGEND			FRAMING	SCHE	DULE
				CALLOUT	JOIST/BEAM	HANGER (U.O.N. ON PLAN)	REFER TO DETAIL(S) (OR SEE NOTES BLW)
	STUD WALL ABOVE FLOOR			MFJ1	11 ⁷ ₈ TJI 210 @16"o.c.	ITS2.06/11.88	A S7
	WALL BELOW FLOOR	BFP	SEE SHEET S3 PLAN LEGEND	MFB2	5½×9 GLB (DROPPED)	N/A	C S7
	WINDOW BY ARCH (S.A.D.)	ISTR	INTERIOR STAIR FRAMING PER S7	MFB3	1 ³ / ₄ ×11 ⁷ / ₈ LVL (FLUSH)	ITS1.81/11.88	N/A
(X) 'L'	½" W.S.P. SHEAR WALL TYPE X H AND DETAIL W/ MIN. LENGTH 'L', PER S2 CALLOUTS ON PLAN	ORP	OUTDOOR ROOF POSTS CONTINUE THROUGH DECK w/ 2x6 PT LEDGER FACE-NAILED TO EACH SIDE w/ (4) 10d TO SUPPORT DECKING	DJ1	2x10 PT @16"o.c.	LUS210Z	E S7
OR ATA	POST ABOVE <u>OR</u> BELOW FLOOR PER <u>E-F C-E</u> <u>S2 S7</u>	SOG	4" CONCRETE SLAB ON GRADE N w/ 6x6-W4.5xW4.5 W.W.F. PER S2	DB2	6x10 PT (DROPPED)	N/A	D S7
* 2 XDV2	POST & HOLDOWN PER L S2	SWO	STRAP AROUND SHEAR WALL WITH OPENINGS PER B S2				
WSWXX ⊠x	SIMPSON STRONG WALL WSWXX PER ATTACHED MANUFACTURER'S DETAIL SHEETS. AT ATTACHMENT TO FOUNDATION, USE 'SB1x30' CIP ANCHORS IN LIEU OF WSW-AB						
		BEAM_ HANGER	FLUSH-FRAMED JOIST OR BEAM CONNECTION; SEE FRAMING SCHEDULE FOR HANGERS, U.O.N. ON PLAN OR DETAILS (JOIST HANGERS NOT SHOWN ON PLAN FOR CLARITY)				
		5	JOIST OR BEAM BEARING ON DROPPED BEAM OR HEADER (BEARING WALL SIM). POST DOWN TO HEADER WHERE OCCURS (POST WIDTH TO MATCH BEAM, NOT SHOWN FOR CLARITY). INSTALL FULL—DEPTH BLKG EACH SIDE OF JOIST OR BEAM OVER SUPPORT				



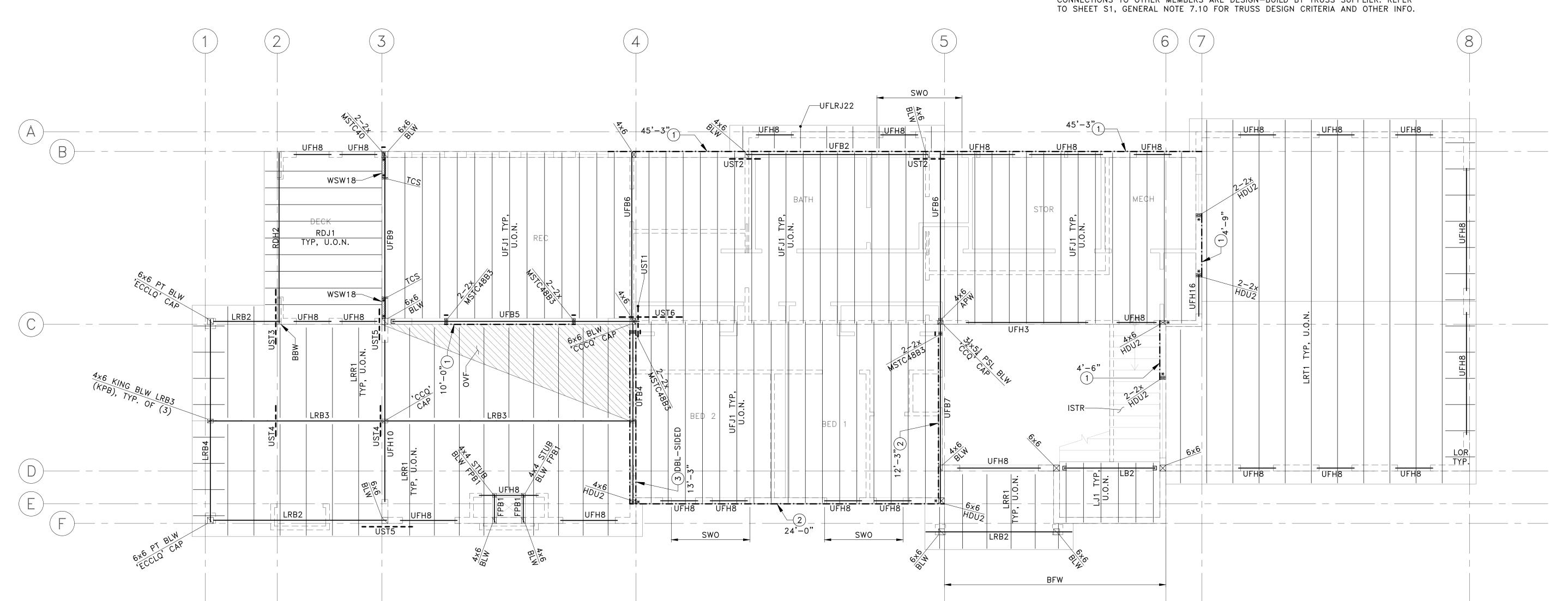


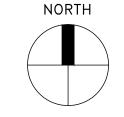
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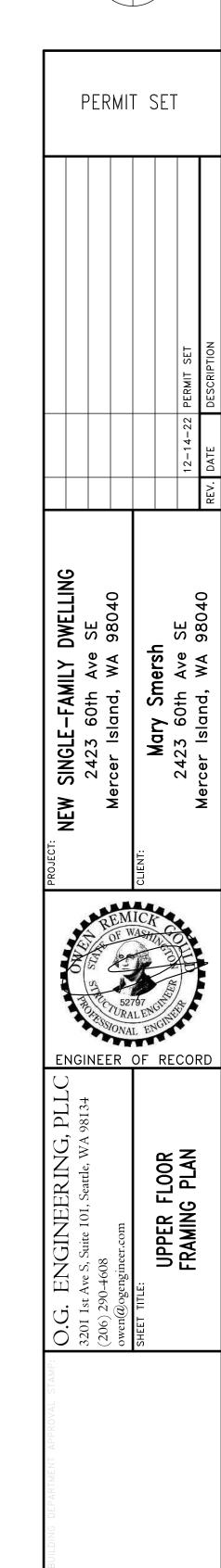


	PLAN L	EGEND			FRAMING	SCHE	DULE		FRAMING	SCHE	DULE
	STUD WALL ABOVE FLOOR	APW	ALIGN SOUTH FACE OF POST W/ SOUTH FACE OF BEARING WALL BELOW	CALLOUT	JOIST/BEAM	HANGER (U.O.N. ON PLAN)	REFER TO DETAIL(S) (OR SEE NOTES BLW)	CALLOUT	JOIST/BEAM	HANGER (U.O.N. ON PLAN)	REFER TO DETAIL(S) (OR SEE NOTES BLW)
	WALL BELOW FLOOR	BBW	BLOCK SOLID BELOW END OF LRB2 DOWN TO T.O. WSW	UFJ1	11 ⁷ ₈ TJI 210 @16"o.c.	IUS2.06/11.88	A S8	LRR1	2x12 @24"o.c.	LRU212Z	D-F S8
	WINDOW BY ARCH (S.A.D.)	BFW	B.F. $1\frac{3}{4} \times 5\frac{1}{2}$ LVL EXTERIOR WALL STUDS @16"o.c. FROM MAIN FLOOR TO ROOF w/ 'A35' T&B @ EACH STUD & 'A35' T&E EACH SIDE OF MULTI-PLY STUDS & POSTS WITHIN WALL	UFB2	$5\frac{1}{4} \times 11\frac{7}{8}$ PSL (FLUSH w/ UFJ1)	N/A	S8	LRB2	5½×9 GLB (DROPPED)	N/A	N S8
X 'L'	½" W.S.P. SHEAR WALL TYPE X H AND DETAIL W/ MIN. LENGTH 'L', PER S2 CALLOUTS ON PLAN	ISTR	INTERIOR STAIR FRAMING PER (S7)	UFH3	4×10 (DROPPED HEADER)	N/A	A S2	LRB3	3½×11½ PSL (FLUSH RIDGE)	N/A	G S8
OR ATA	POST ABOVE <u>OR</u> BELOW FLOOR PER <u>S2</u> U.O.N.	КРВ	KING POST BEARS ON T.O. DROPPED BEAM w/ INV. 'AC' BASE	UFB4	$5\frac{1}{4} \times 11\frac{7}{8}$ PSL (FLUSH w/ UFJ1)	N/A	S8	LRB4	5½×9 GLB (DROPPED)	N/A	N/A
2 17 OR 2 2 15 C 2 8	POST & HOLDOWN OR TIEDOWN STRAP PER S2	LOR	LOOKOUT RAFTER PER E B S9	UFB5	$5\frac{1}{4} \times 11\frac{7}{8}$ PSL (FLUSH w/ UFJ1)	HHUS5.50/10	CONTINUE EAST END OF UFB5 MIN. 3" EAST OF 6x6 POST BLW	LRT1*	COMMON GABLE TRUSSES @24"o.c.	N/A	A-B S9
OVF	OVER-FRAMING PER K S8	SWO	STRAP AROUND SHEAR WALL WITH OPENINGS PER B S2	UFB6	$3\frac{1}{2} \times 11\frac{7}{8}$ LSL (FLUSH w/ UFJ1)	N/A	E.N. FLOOR SHEATHING TO FULL LENGTH OF UFB6	FPB1	4x6 (AT BOTTOM OF UPPER CHIMNEY CHASE WALLS)	N/A	S8
WSWXX ≸‱x	SIMPSON STRONG WALL WSWXX PER ATTACHED H MANUFACTURER'S DETAIL SHEETS AND S8	TCS	$1\frac{3}{4} \times 5\frac{1}{2}$ LVL CRIPPLE STUD (SUPPORTS TRANSOM HEADER ABOVE) w/ 16d@6"o.c. FACE-NAIL TO SIDE OF WSW	UFB7	$3\frac{1}{2} \times 11\frac{7}{8}$ PSL (FLUSH w/ UFJ1)	N/A	B-C S8				
	METAL STRAP PER PLAN (PLACE o/ SHEATHING WHERE OCCURS). E.N. ROOF/ FLOOR SHEATHING TO FULL LENGTH OF ATTACHED BEAMS & JOISTS WHERE OCCURS			UFH8	4x8 (DROPPED HEADER)	N/A	A S2				
UST1	'MSTC40' STRAP o/ T.O. BEAM TO ABUTTING BEAM			UFB9	$5\frac{1}{4} \times 11\frac{7}{8}$ PSL (FLUSH w/ UFJ1)	N/A	H,L OF UFB9 MIN. 6" SOUTH END OF 6x6 POST BLW				
UST2	'MSTC40' STRAP U/S BEAM TO I T.O. ABUTTING TOP PLATE PER S8			UFH10	$5\frac{1}{4} \times 9\frac{1}{2}$ PSL (HEADER BTWN DOOR & TRANSOM WINDOWS)	N/A	A S2 NSTALL UFH8 ABOVE TRANSOM WINDOWS				
UST3	'CS16' STRAP 18" o/ LRR1 & CONTINUE MIN. 36" o/ BLKG BTWN UDJ1 (ADD LRR1 AS REQ'D TO ALIGN)	BEAM_ HANGER	FLUSH-FRAMED JOIST OR BEAM CONNECTION; SEE FRAMING SCHEDULE FOR HANGERS, U.O.N. ON PLAN	RDJ1	$1\frac{3}{4} \times 9\frac{1}{2}$ LVL @16"o.c. (RIP TO SLOPE, S.A.D.)	HU7	C-M DEPTH AT LOW END OF SLOPE				
UST4	'MSTA36' STRAP o/ LRR1 ATTACHED TO UST3&5 ACROSS RIDGE TO ABUTTING LRR1 (ADD LRR1 AS REQ'D TO ALIGN w/ STRAPS)	٨	OR DETAILS (JOIST HANGERS NOT SHOWN ON PLAN FOR CLARITY)	RDH2	5½×9¼ PSL (DROPPED)	N/A	L CONTINUE RDB2 ALL S8 THE WAY OVER WSWs				
UST5	'MSTA36' STRAP o/ LRR1 & UFB9 (ADD LRR1 AS REQ'D TO ALIGN)	7	JOIST OR BEAM BEARING ON DROPPED BEAM OR HEADER (BEARING WALL SIM). POST DOWN TO HEADER								
UST6	'CS16' STRAP 18" o/ UFB5 & CONTINUE o/ $1\frac{1}{2}$ LSL BLKG BTWN UFJ1 FOR MIN (3) BAYS w/ (2) 'A35' CLIPS @ BLKG TO TOP PLATE BLW EACH BAY, MIN. (6) CLIPS TOTAL	<u></u>	WHERE OCCURS (POST WIDTH TO MATCH BEAM, NOT SHOWN FOR CLARITY). INSTALL FULL—DEPTH BLKG EACH SIDE OF JOIST OR BEAM OVER SUPPORT						DIATE CONNECTED WOOD I		

*ALL METAL-PLATE CONNECTED WOOD TRUSSES, STRUCTURAL FASCIAS AND THEIR CONNECTIONS TO OTHER MEMBERS ARE DESIGN-BUILD BY TRUSS SUPPLIER. REFER TO SHEET S1, GENERAL NOTE 7.10 FOR TRUSS DESIGN CRITERIA AND OTHER INFO.







SHEET. NO.

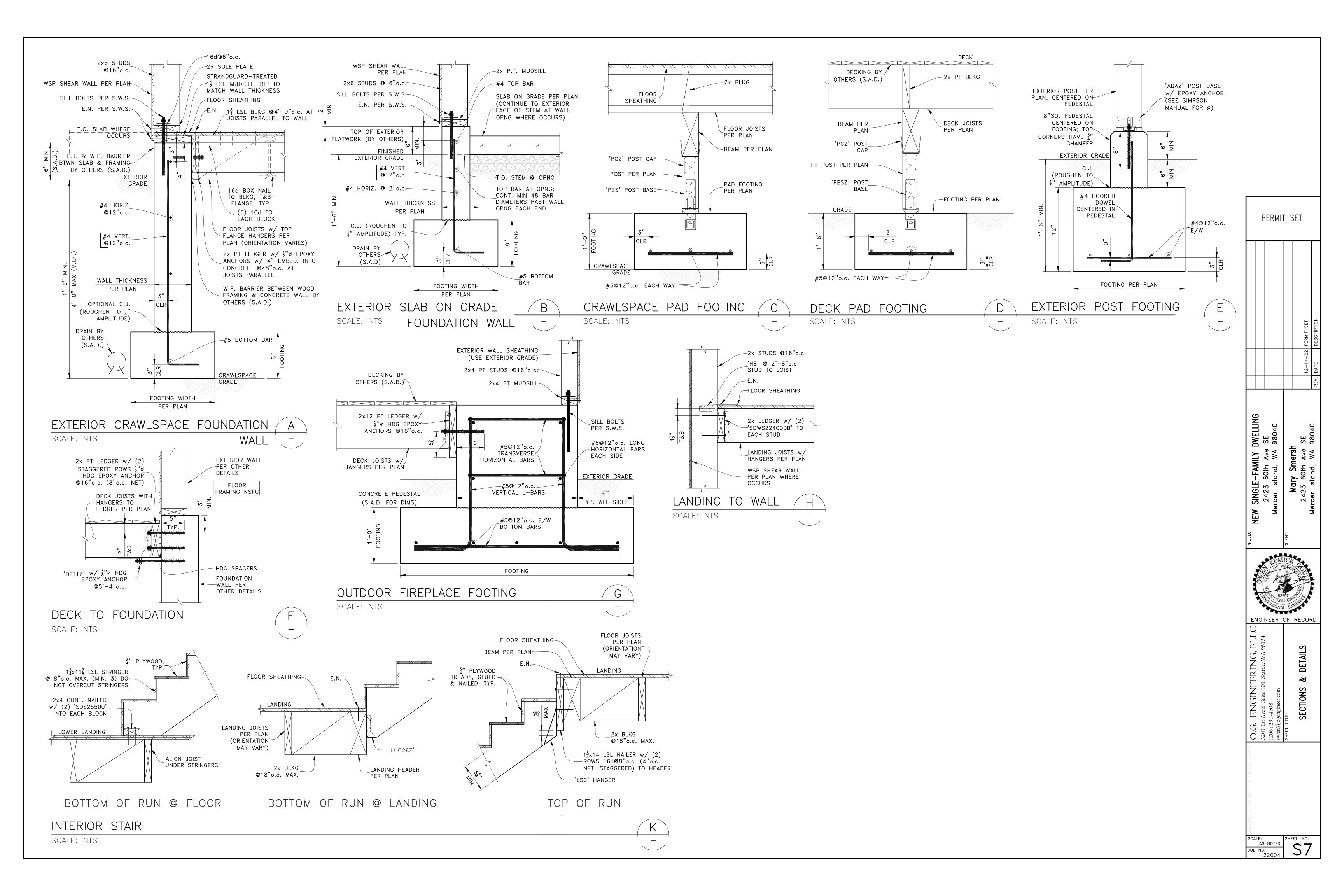
	PLAN L	LGENU T		SCHEDULE THANGER DEFER TO DETAIL (ST	<u> </u>		
	STUD WALL ABOVE ROOF		CALLOUT JOIST/BEAM COMMON GABLE	HANGER REFER TO DETAIL(S) (U.O.N. ON PLAN) (OR SEE NOTES BLW)			
	WALL BELOW ROOF F-F		RT1* COMMON GABLE TRUSSES @24"o.c. COMMON GABLE	N/A $A-B$ $S9$			
BLW	POST BELOW ROOF PER E-F S2		RT2* COMMON GABLE TRUSSES @24"o.c.	BY SUPPLIER A-B S9 UNIFORM TOP CHORD	D LOAD = 250 PLF		
	OVER-FRAMING PER K S8 METAL STRAP PER PLAN (PLACE o/ SHEATHING WHERE		RT3* GABLE DRAG TRUSS	N/A B UNIFORM TOP CHORD CONCENTRATED BOTT AT RST1 = 6000 LB	OM CHORD LOAD BS		NORTH
	OCCURS). E.N. ROOF SHEATHING TO FULL LENGTH OF ATTACHED BEAMS & JOISTS WHERE OCCURS		RR4 2x12 @24"o.c.	LRU212Z D-È S8			
RST1	'MSTC40' STRAP U/S DRAG TRUSS TO T.O. TOP PLATE, E.N. ROOF SHEATHING TO FULL S8 SIM LENGTH OF DRAG TRUSS TOP CHORD PER		RB5 $3\frac{1}{2} \times 11\frac{7}{8} \text{ PSL}$ (FLUSH RIDGE) $3\frac{1}{2} \times 9\frac{1}{2} \text{ PSL} \text{ (HEADER)}$	N/A G S8 HUCQ410 BEARS ON T.O. TCS & HANG	es l		
RST2	'MSTC40' STRAP U/S BEAM TO I T.O. ABUTTING TOP PLATE PER S8		RH6 RH6 S12×91/2 PSL (HEADER BTWN DOOR & TRANSOM WINDOWS)	HUCQ410 (TO SIDE OFF WSW. INSTALL RH7 OF WSW) ABOVE TRANSOM WINDOWS.			
RST3	'MSTA30' STRAP T.O. DROPPED BEAM TO T.O. ABUTTING TOP PLATE		RH7 4x8 (DROPPED EXTERIOR HEADER) 3\frac{1}{2}x11\frac{7}{8} PSL (UPSET.	N/A BEVEL TOP OUTSIDE			PERMIT SET
			RB8 $3\frac{1}{2} \times 11\frac{7}{8}$ PSL (UPSET, U/S FLUSH w/PLATE HEIGHT) $5\frac{1}{2} \times 7\frac{1}{2}$ GLB (DROPPED, TOP FLUSH w/PLATE	N/A BEVEL TOP OUTSIDE CORNERS 3" MAX. AS REQ'D TO FIT IN EAVES			
LOR	LOOKOUT RAFTER PER E B S9		RB9 TOP FLUSH w/ PLATE HEIGHT)	N/A N/A			
РОВ	POST BEARS ON T.O. BEAM w/ INV. 'AC' BASE						
TCS	$1\frac{3}{4} \times 5\frac{1}{2}$ LVL CRIPPLE STUD (SUPPORTS TRANSOM HEADER) w/ 16d@6"o.c. FACE-NAIL TO SIDE OF WSW	BEAM					
		OR DETAILS (JOIST HANGERS NOT SHOWN ON PLAN FOR CLARITY)					IT SET
		JOIST OR BEAM BEARING ON DROPPED BEAM OR HEADER (BEARING WALL SIM). POST DOWN TO HEADER WHERE OCCURS (POST WIDTH TO MATCH BEAM, NOT					22 PERM
		SHOWN FOR CLARITY). INSTALL FULL—DEPTH BLKG EACH SIDE OF JOIST OR BEAM OVER SUPPORT					12-14-; DATE
			CONNECTIONS TO OTHER MEMBERS ARE	TRUSSES, STRUCTURAL FASCIAS AND THEIR DESIGN—BUILD BY TRUSS SUPPLIER. REFER R TRUSS DESIGN CRITERIA AND OTHER INFO.			REV.
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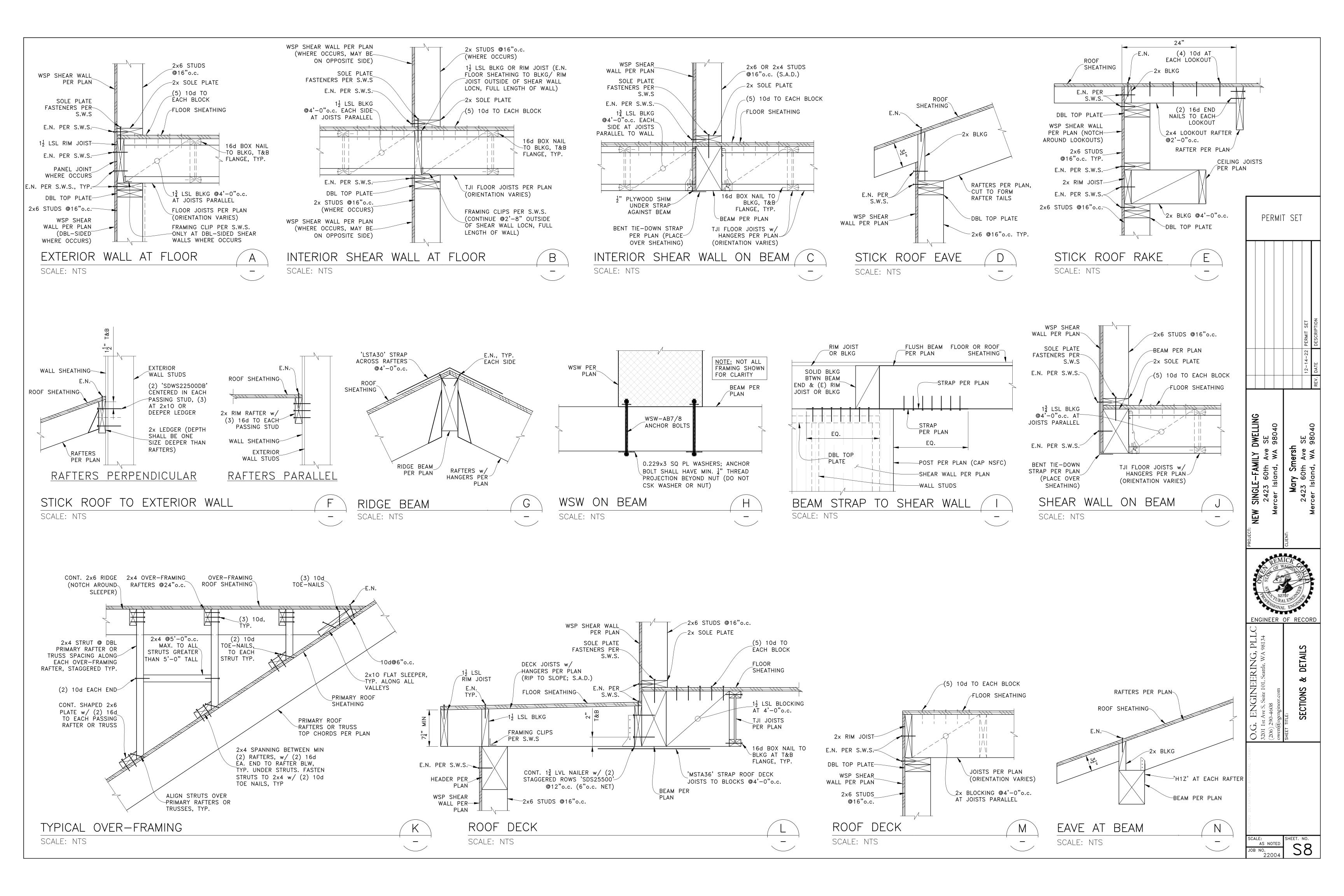
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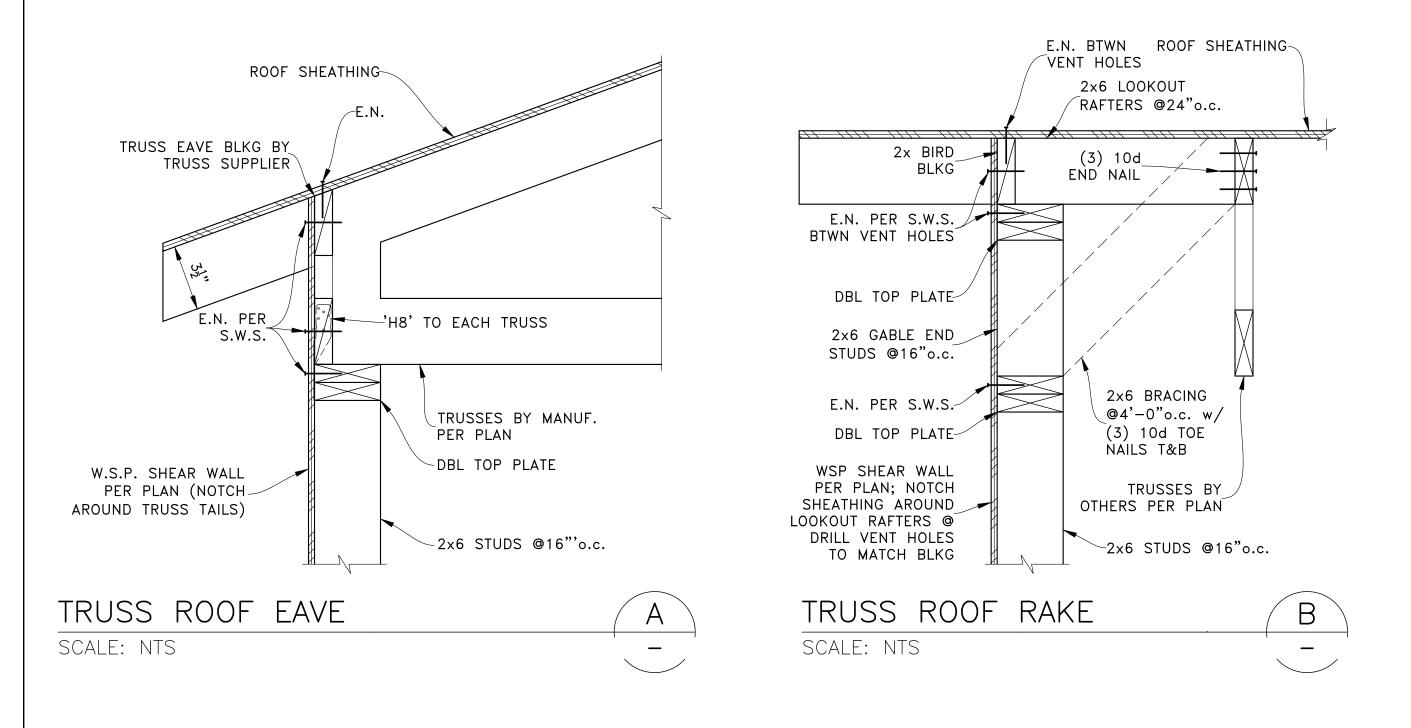
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NEW SINGLE—FAMILY DWELLING

2423 60th Ave SE

Mercer Island, WA 98040

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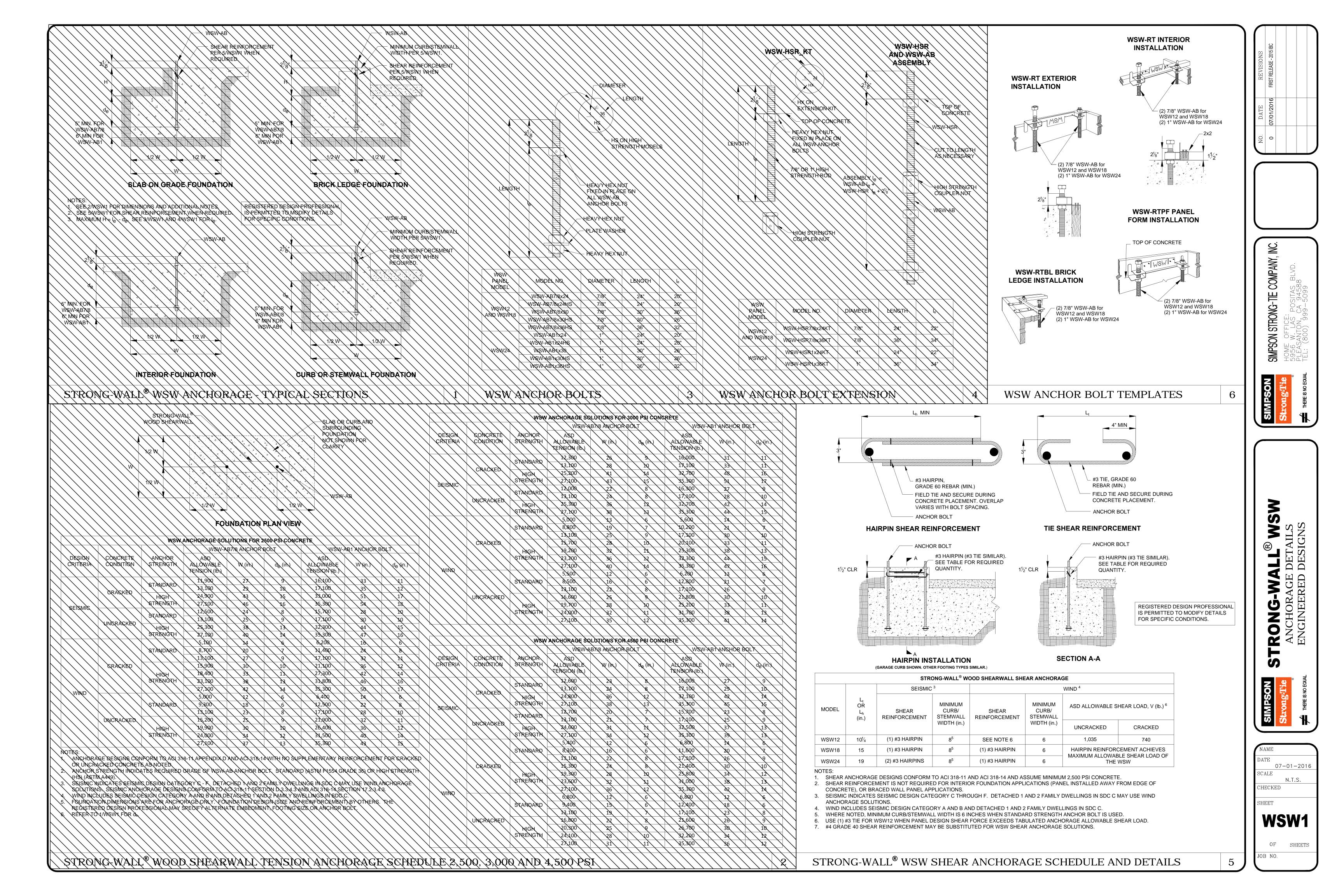
Mary Smersh

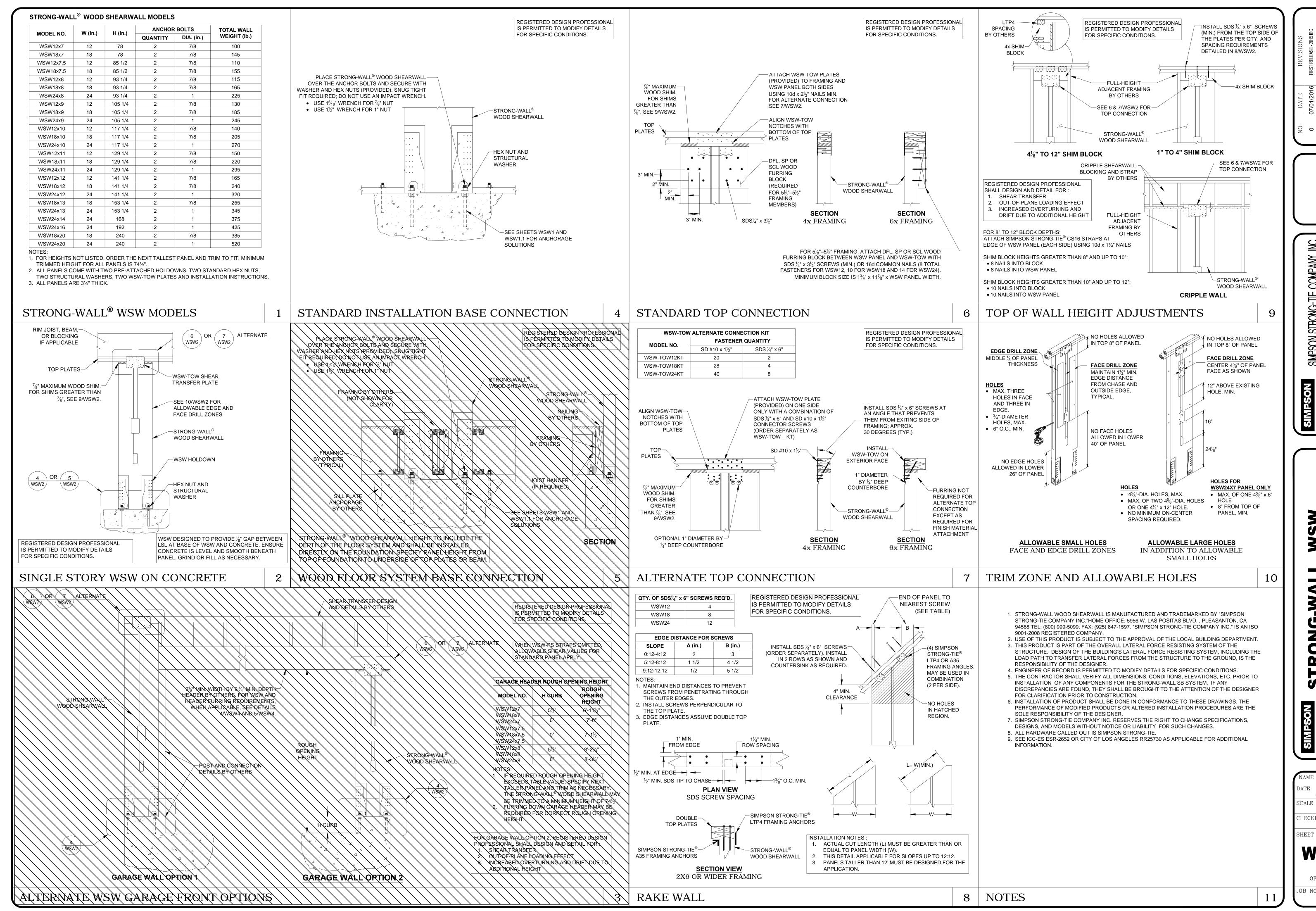
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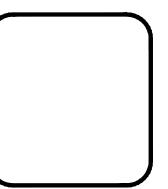
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Mercer Island, WA 98040

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COMPANY,

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07-01-2016 N.T.S. CHECKED

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